

Ref: 8102

October 2, 2024

Mr. Takashi Tada Land Use Director/Town Planner Town of Groton Planning Board 173 Main Street Groton, MA 01450

Re: Response to Traffic Engineering Review

Gratuity Brook Farm Estates – 63 Gratuity Road

Groton, Massachusetts

Dear Takashi:

Vanasse & Associates, Inc. (VAI) is providing responses to the comments that were raised in the September 10, 2024 *Traffic Engineering Review* letter prepared by Nitsch Engineering (Nitsch) concerning their review of the April 23, 2024 *Transportation Impact Assessment* (the "April 2024 TIA") that was prepared by VAI in support of the proposed Gratuity Brook Farm Estates residential development to be located at 63 Gratuity Road in Groton, Massachusetts (hereafter referred to as the "Project"). Listed below are the comments that were identified by Nitsch in the subject letter followed by our response on behalf of the Project proponent.

Comment 1:

The memorandum references that the Project will be accessed off both Gratuity Road and Jenkins Road, and references the access points as "Project Site Roadway" for each. The plans reference the Project Site Roadway for Gratuity Road as "Gerard Way," and the Project Site Roadway for Jenkins Road as "Therese Lane." We recommend VAI clarify the intended nomenclature for the Project Site Roadways and reference the intended names in future correspondence.

Response:

Within this letter and in all future correspondence, the Project site roadways that will intersect Gratuity Road and Jenkins Road will be referred to as "Gerard Way" and "Therese Lane", respectively.

Comment 2:

The memorandum states that off-street parking will be provided in individual garages and driveways that will accommodate a minimum of two (2) vehicles per dwelling. We recommend VAI clarify parking requirements from the Town of Groton, and present information from the Institute of Transportation Engineers' (ITE's) Parking Generation to establish if the Project would provide sufficient parking.

Response:

Section 218-8.1, *Off-street parking and loading*, of the Zoning Bylaw of the Town of Groton requires a minimum of 2.0 parking spaces per unit be provided for single or two-family dwellings containing more than one (1) bedroom.

Parking demand data published by the Institute of Transportation Engineers (ITE)¹ for Land Use Code (LUC) 215, Single-Family Attached Housing, indicates that the observed peak parking demand ranges between 0.90 and 2.55 parking spaces per dwelling unit, with an average observed peak parking demand of 1.41 parking spaces per unit and an 85th percentile observed peak parking demand² of 2.27 parking spaces per unit. Parking demands for an age-restricted residential community are generally less than those for a conventional (non-age restricted) community.

The proposed parking supply (a minimum of 2.0 parking spaces per unit) meets the parking requirements of the Zoning Bylaw of the Town of Groton and is consistent with the parking ratios observed by the ITE for a single-family (non-age qualified) residential community.

Comment 3:

Based on our site visit, the Gratuity Road pavement quality is poor in some areas between the future Site driveway and Mill Street with cracking and potholes. We recommend VAI confirm the pavement condition and clarify if pavement resurfacing is proposed as part of the Project.

Response:

VAI agrees that the pavement along Gratuity Road is in fair to poor condition. The resurfacing of Gratuity Road is not currently proposed as a part of the Project outside of the areas where the construction of the Project site roadways and the installation or upgrade of utilities may be required to accommodate the Project.

Comment 4:

The memorandum indicated that no lighting was present on Gratuity Road near Mill Street. Based on our site visit, we note street lighting is present. We recommend VAI clarify if lighting was recently installed on Gratuity Road, and if there are plans independent of the Project to install additional lighting within the study area.

Response:

The Groton Electric Light Department (GELD) confirmed that the installation of two (2) pole mounted lights in the vicinity of private driveways requested by residents of Gratuity Road have occurred within the past year. GELD also confirmed that there are not lighting projects planned within the study area at this time.

Comment 5:

Based on our site visit proximate to the intersection of Main Street at Mill Street, there is an existing sign that indicates a heavy vehicle restriction on Mill Street. We recommend VAI confirm that the truck restriction is present, clarify whether there are additional heavy vehicle restrictions within the study area, and how the restrictions may impact safety and operations within the study area as a result of the Project.

¹Trip Generation, 6th Edition; Institute of Transportation Engineers; Washington, DC; 2023.

²The 85th percentile peak parking demand is defined as the parking demand at which 85 percent of the observed peak parking demands fall below and 15 percent are above.

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Response:

VAI has confirmed that per Section 266-23, *Vehicle weight restrictions*, of the Zoning Bylaw of the Town of Groton, the use and operation of heavy commercial vehicles having a carrying capacity of more than 2.5 tons is restricted along Mill Street. No other heavy vehicle restrictions are present within the study area.

Contractors working on the Project will be informed of the weight restriction on Mill Street. Post-construction truck traffic associated with the Project is expected to be consistent with the truck traffic (and likely the same vendors serving the area) that serves the existing residential homes in the area and primarily vehicles that are less than 2.5 tons (i.e., UPS, FedEx, Amazon, etc.).

Comment 6:

Based on our site visit, we observed the following at the intersection of Main Street at Mill Street:

- a. There are no Americans with Disabilities Act (ADA) and Massachusetts Architectural Access Board (MAAB)-compliant pedestrian ramps or crosswalks present across Mill Street, despite a sidewalk on the west side of Main Street.
- b. The stop line for the Mill Street eastbound approach is placed beyond the projection of the pedestrian desire line for the sidewalk on the west side of Main Street.
- c. The available sight distance looking left (southbound) and looking right (northbound) from the Mill Street eastbound approach is impeded by a berm to the south and vegetation to the north. As a result, we observed vehicles inching from the stop line closer to Main Street and into the pedestrian desire line to obtain better observation of approaching vehicles along Main Street.
- d. The curb radii on both corners of Mill Street are significant given that heavy vehicles are restricted along Mill Street, which can promote high turning speeds for the free southbound right turn and lengthen the crossing distance for pedestrians traveling on the west side of Main Street.

We recommend VAI consider working with the Town to facilitate a proposed improvement plan that may provide better accessibility for vehicles and pedestrians at the intersection of Main Street at Mill Street.

Response:

The Project proponent will relocate the stop-line on the Mill Street approach and trim/remove vegetation located within the public-right-of-way on the corners of the intersection in order to improve sight lines. In addition, the Project proponent will coordinate with the Town to advance additional improvements at the intersection that are commensurate with the impact of the Project.

Comment 7:

Table 1 indicates that the two intersections along Main Street have bicycle accommodations via a combined shoulder and travel lane width equal to or exceeding 14 feet. Though note "b" is not shown for the intersection of

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Main Street at Arlington Street, we consider that note "b" is applicable for this location in addition to Main Street at Mill Street. Based on our field observations, the travel lanes are not designated as shared lanes by signage or Sharrow pavement markings. We recommend VAI clarify the presence of bicycle accommodations though no signage or Sharrow pavement markings are present and note any recommendations to provide additional bicycle accommodations within the study area.

Response:

VAI agrees that a shared-traveled-way is provided along Main Street in the vicinity of Arlington Street. Independent of an unrelated to the Project, "Share the Road" signs (graphic symbol) could be installed along Main Street (Route 119) to the extent so desired by the Town and as approved by MassDOT.

Comment 8:

The discussion of seasonal factors on page 5 and presented in the Appendix reviews the factors for Urban Groups 4-7. However, Main Street is classified as an urban principal arterial, which falls under Urban Group 3. We recommend VAI clarify the functional classification of all the roadways included within the study area and establish if seasonal factors should be applied to additional traffic volumes within the study area.

Response:

With the exception of Main Street, all of the study area roadways are classified as local roadways under Town jurisdiction. Main Street within the study area is classified as an urban principal arterial under MassDOT jurisdiction. A review of MassDOT weekday seasonal factors for Urban Group 3 (other principal arterials) and Urban Group 4-7 (minor arterials, major and minor collectors and local roads and street)³ indicates that traffic volumes during the month of March are approximately 2.0 and 5.3 percent above average-month conditions, respectively. As such, and consistent with the methodology presented in the April 2024 TIA, a seasonal adjustment was not applied to the March 2024 traffic counts as they are representative of above average-month conditions.

Comment 9:

The ATR data sheets in the Appendix for the Gratuity Road location indicate the counts were collected east of Mill Street, not west of Mill Street. We recommend VAI clarify the correct ATR collection.

Response:

The ATR counts were conducted on Gratuity Road west of Mill Street.

Comment 10:

The memorandum notes that the peak-hour volumes presented in Table 2 for Gratuity Road and Jenkins Road are obtained from information provided in Figures 2 and 3. We note the following:

a. The peak periods and ATR information between Table 2 and Figures 2 and 3 are inconsistent with the information provided in the Appendix.

³MassDOT statewide Traffic Data Collection; 2019 Weekday Seasonal Factors, Groups U3 and U4-7.



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b. The K-factors presented in Table 2 may be unreasonably high given the roadway classifications within study area. The method for how the K-factors were calculated is also unclear.

VAI should clarify the information from Table 2, Figures 2 and 3, and provide revised information, if needed.

Response:

The average weekday traffic volumes presented in Table 2 are consistent with the "week average" traffic volume in the right-most column of the ATR count sheet that averages traffic per direction.

The peak-hour traffic volumes presented in Table 2 are consistent with the 2024 Existing traffic volumes depicted on Figures 2 and 3, which are consistent with the turning movement count data that is presented in the Appendix.

The K-factors in Table 2 represent the percent of the average weekday traffic that occurs during the respective peak hour (i.e., the Gratuity Road weekday morning peak-hour K-factor is calculated as follows: 25 vehicles per hour ÷ 165 daily vehicles = 15.2 percent). VAI agrees that the calculated K-factors are generally higher than the standard K-factors observed for many local roadways (typically in the range of 9 to 10 percent) and is a function of the relatively low traffic volumes along the study area roadways.

Comment 11:

The ATR counts for Gratuity Road, west of Mill Street, and Jenkins Road, east of Hickory Drive presented within the Appendix do not include vehicle classifications. Particularly, the heavy-vehicle percentages on the two roadways with ATR counts are of importance so we can understand the potential impact of heavy vehicles within the study area. We recommend VAI clarify if heavy vehicle data is available, and if not, consider collecting an additional ATR counts so we may understand the impact of heavy vehicles within the study area.

Response:

Heavy vehicle data was collected at the study area intersections in conjunction with the TMC data and, with the exception of Main Street, was found to be two (2) heavy vehicles or less per movement during the peak periods. Due to the existing heavy vehicle restriction along Mill Street, heavy vehicle activity within the study area outside of Main Street is generally limited. Outside of the construction period, the Project will not result in a material increase in truck traffic.

Comment 12:

In Table 2, the Average Weekday Traffic (AWT) presented for Gratuity Road and Jenkins Road may be different than the calculated average between the two sets of 24-hour counts. We recommend VAI clarify the calculation methodology, confirm if the numbers presented in Table 2 are correct, and if not, present the correct information.

Response:

See response to Comment #10.

Comment 13:

The Pedestrian Facilities discussion on page 9 notes pedestrians were observed crossing at the Main Street/Mill Street and Mill Street/Gratuity Road/Arlington Street intersections during the peak hours, and that pedestrian activity



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along the study area roadways was found to be generally low. With implementation of the Project, there is potential for additional pedestrian activity within the study area. We recommend VAI clarify when the pedestrian observations were collected, including time of year and time of day, to understand if there is potential for additional pedestrian activity during other times not observed.

Response:

Pedestrian activity at the study area intersections was observed in conjunction with the TMC data collected in March 2024 during the weekday morning (7:00 to 9:00 AM) and evening (4:00 to 6:00 PM) peak periods, which is consistent with time periods that are evaluated for a residential development in a similar setting.

Comment 14:

In the crash rate worksheets presented in the Appendix, we present the following:

- a. It is unclear whether the AM or PM peak-hour volumes were used to calculate crash rates;
- b. A K-factor of 0.097 is used at all intersections, which is consistent with the count data from the previous TIA, but is not consistent with the K-factors presented in Table 2;
- c. The worksheet for the Main Street/Arlington Street intersection indicates one (1) crash occurred; however, Table 4 indicates no crashes at this location.

We recommend VAI present updated crash rate worksheets, and update Table 4, if needed.

Response:

The MassDOT crash rate worksheets that were provided in the Appendix of the April 2024 TIA were inadvertently obtained from a prior assessment that was performed for the Project. As correctly presented in Table 4 of the April 2024 TIA, a review of motor vehicle crash data for the 2017 through 2021 review period indicates that no (0) motor vehicle crashes were reported to have occurred at the Main Street/Arlington Street intersection.

Updated intersection crash rate worksheets are attached which: i) denote the peak hourly volumes (either AM or PM) to the left of numerical peak hour volumes; and ii) use the MassDOT standard K-factor of 0.09, as the calculated K-factors in Table 2 are not representative of traffic volumes along Main Street or along the Mill Street approaches to Gratuity Road or Jenkins Road, as they depict traffic volumes along local side streets that serve mainly local resident traffic. These revisions did not result in a material change from the information that is presented in Table 4 of the April 2024 TIA.

Comment 15:

We coordinated with the Town to confirm the Specific Development by Others presented on page 10. Based on coordination with the Town, construction is underway for the Proposed Residential Development, Hayes Woods Road; however, the Town does not anticipate any occupancy permits being issued in 2024. The Proposed Age-Restricted Multifamily Residential Development at

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797 Boston Road was permitted by the Planning Board in June 2024. Based on the memorandum, the traffic volumes associated with this development within the study area are expected to be relatively minor and would be included in the general background traffic growth rate. We obtained the Special Permit Application for the 797 Boston Road project, and the application states that this project is anticipated to generate 78 vehicle trips per day, five (5) vehicle trips during the weekday morning peak hour, and six (6) vehicle trips during the weekday evening peak hour. We recommend VAI clarify if the specific traffic information provided in the Special Permit Application for 797 Boston Road would significantly impact the results presented in the memorandum.

Response:

The minor traffic volumes associated with the 797 Boston Road development project (between five (5) and six (6) vehicle trips during the peak hours) would not result in material increase in peak-hour traffic to the extent that there would be an increase in average motorist delay at the study area intersections that would change the analysis results or the findings that are presented in the April 2024. We note that the 797 Boston Road property is located approximately 4.1 miles from the closest study area intersection (Mill Street at Jenkins Road) and that the majority of the associated trips will likely be on Main Street.

Comment 16:

In Figure 5 – 2031 No Build Weekday Evening Peak Hour Traffic Volumes, the Mill Street southbound volume at the intersection of Mill Street at Arlington Street and Gratuity Road should be consistent with the 2024 Existing Weekday Evening Peak Hour Traffic Volumes, presented in Figure 3, grown by the background growth rate, and presented in the Synchro analysis in the Appendix. Based on our understanding, the volume should be 57 vehicles instead of 87 vehicles. We recommend VAI clarify if the 57 vehicles should be the correct number of vehicles presented in Figure 5.

Response:

Figure 5R is attached and has been revised to show 57 through vehicles on the Mill Street southbound approach to the Mill Street/Arlington Street/Gratuity Road intersection. The 2031 No-Build and Build condition traffic operations analysis results that are presented in the April 2024 TIA correctly incorporate 57 vehicles for this movement.

Comment 17:

In Table 5, Column B, based on the information from the Appendix, the number of entering vehicles should be four (4) and the number of exiting vehicles should be three (3). We note that the seven (7) total vehicles presented is consistent with information from the Appendix. We recommend that VAI clarify the numbers presented in Table 5.

Response:

Table 5R has been revised to reflect the correct distribution of entering and exiting trips for ITE LUC 252, *Senior Adult Housing*, during the weekday evening peak-hour of the adjacent roadway: four (4) entering vehicles and three (3) exiting vehicles. This revision does not impact the analyses or findings that are presented in the April 2024 TIA which was based on the use of the higher traffic volumes that are derived using ITE LUC 215, *Single-Family Attached Housing*.



Table 5R TRIP-GENERATION SUMMARY

Time Period/Direction	(A) Single-Family Attached Housing ^a	(B) Multifamily Senior Adult Housing ^b
Average Weekday Daily:		
Entering	82	46
<u>Exiting</u>	<u>82</u>	<u>46</u>
Total	164	92
Weekday Morning Peak Hour:		
Entering	2	2
<u>Exiting</u>	<u>7</u>	<u>4</u> 6
Total	9	6
Weekday Evening Peak Hour:		
Entering	8	4
Exiting	<u>_5</u>	<u>3</u> 7
Total	<u>5</u> 13	$\overline{7}$

^aBased on ITE LUC 215, Single-Family Attached Housing (28 units).

Comment 18:

Based on information from ITE's Trip Generation, 11th edition, the TIS used Land Use Code (LUC) 215–Single Family Attached Housing and LUC 252 – Senior Adult Housing for the Trip Generation Summary presented in Table 5. Based on information available from Trip Generation, the Project may fit the description of LUC 251, Single-Family Senior Adult Housing, better than either LUC 215 or LUC 252. According to the ITE description, LUC 251 includes both detached and attached dwelling units, including "traditional single-family detached homes, patio homes, duplexes, and townhouses. Single-family attached housing includes any single-family housing unit that shares a wall with an adjoining dwelling unit, whether the walls are for living space, a vehicle garage, or storage space." LUC 252 is used for multifamily units that "share both floors and walls with other units in the residential building." We recommend VAI evaluate the potential to apply LUC 251 and establish how this may impact the 2031 Build Volumes and Analysis results, Summary, and Recommendations.

Response:

Table 5A summarizes the trip characteristics of the Project using ITE LUC 251, Single-Family Senior Adult Housing applied to 28 units.



^bBased on ITE LUC 252, Senior Adult Housing – Multifamily (28 units).

Table 5A TRIP GENERATION SUMMARY

		Vehicle Trips ^a	
Time Period	Entering	Exiting	Total
Average Weekday:	61	61	122
Weekday Morning Peak-Hour:	2	5	7
Weekday Evening Peak-Hour:	5	3	8

^aBased on ITE LUC 251, Single-Family Senior Housing (28 unit).

As can be seen in Table 5A, using ITE LUC 251, Single-Family Senior Housing, the Project is expected to generate approximately 122 vehicle trips on an average weekday (two-way, 24-hour volume) with 7 vehicle trips expected during the weekday morning peak-hour and 8 vehicle trips expected during the weekday evening peak-hour.

Based on a review of Tables 5R and 5A, the traffic volume projections for the Project obtained using ITE LUC 215, *Single-Family Attached Housing*, as presented in the April 2024 TIA continues to provide conservative (high) traffic volumes from which to assess the potential impact of the Project on the transportation infrastructure.

Comment 19:

Gratuity Road is described as 18 feet wide and contains two (2) full traffic lanes. Using the Trip Generation Summary presented in Table 5, the Project estimates 164 added trips per day and approximately 59 trips would use Gratuity Road. The 59 trips would represent an approximately 35% increase in daily trips on Gratuity Road. We recommend VAI recommend additional improvements as part of the Project, including but not limited to: widening Gratuity Road, adding centerline pavement markings, investigate installing speed limit signs, repairing pavement sections that are in poor condition, and/or implement heavy vehicle restrictions.

Response:

The average daily traffic volume along Gratuity Road does not meet the standards or guidance that are provided in the Manual on Uniform Traffic Control Devices (MUTCD)⁴ for the installation of centerline pavement markings and the installation of speed limit signs cannot be installed without the approval of a Special Speed Regulation by MassDOT. In addition, heavy vehicle restrictions also require approval by MassDOT after the completion of an engineering study to justify the restriction and the identification of an alternate truck route that is located entirely within the requesting community. The Project proponent will undertake the resurfacing of Gratuity Road in areas where the construction of the Project site roadways and the installation or upgrade of utilities may be required to accommodate the Project.

⁴Manual on Uniform Traffic Control Devices (MUTCD); Federal Highway Administration; Washington, D.C.; 2009.

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Comment 20:

The Traffic Operations Analysis presented on page 13 states that the peak-hour factors (PHFs) and heavy vehicle percentages (%HV) were obtained from the 2024 TMCs for each approach. However, the Synchro output sheets indicate that the heavy vehicle percentages were applied by movement. We recommend VAI clarify which method was intended to ensure consistency between the Synchro inputs and the narrative.

Response:

The traffic operations analysis presented in the April 2024 TIA used the average peak-hour factor by approach and the heavy vehicle percentage by movement at each study area intersection.

Comment 21:

For the Synchro model associated with the Traffic Operations Analysis, we note the following:

- a. Inputs are inconsistent between the traffic volume figures and the traffic counts included in the Appendix for the following movements:
 - Build Weekday Morning volume for the northbound right-turn movement at Mill Street & Gratuity Road/Arlington Street;
 - Weekday Morning heavy vehicle percentage for NBT at Mill Street & Jenkins Road; and
 - Weekday Morning heavy vehicle percentage for NBR at Jenkins Road & Hickory Drive;
- b. No conflicting pedestrians or approach grades are provided in the inputs, which could impact the overall results. The memorandum indicates that pedestrians are present during the peak hours; therefore, we anticipate the number of conflicting pedestrians to be greater than zero. Based on our site visit, we anticipate several intersection approach grades to be greater than zero; and
- c. Delay for the Weekday Evening Build condition on the eastbound lane at the Main Street at Arlington Street intersection shown in Table 6 is inconsistent with the Synchro analysis.

We recommend VAI confirm the inputs and results shown above and provide an updated analysis to understand the Project impact.

Response:

The traffic operations analysis for 2024 Existing, 2031 No-Build and 2031 Build conditions have been revised to incorporate corrected volumes and heavy vehicle percentages for the above noted movements as well as the inclusion of conflicting pedestrian volumes at the Main Street/Mill Street, Main Street/Arlington Street and Mill Street/Arlington Street/Gratuity Road intersections, the locations where pedestrian activity was observed. Specific approach grade information is not currently available for the study area intersections. Per Exhibit 20-5 of the Highway Capacity Manual 6th Edition, ⁵ a default value of zero (0) percent should be used such cases where

⁵Highway Capacity Manual 6th Edition, Transportation Research Board; Washington, DC; 2016; Page 20-10.



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intersection-specific information is not available. That being said, the approach grades to the study area intersections are gradual and would not impact the capacity of the intersections.⁶ The updated capacity analysis for each condition is summarized in Table 6A.

As can be seen in Table 6A and consistent with the results of the April 2024 TIA, the impact of the Project on the study area is generally defined as a predicted increase in average motorist delay of less than 1.0 seconds that resulted in a corresponding increase in vehicle queuing of up to three (3) feet.

Comment 22:

Table 6 indicates that the Mill Street eastbound approach to the intersection of Main Street at Mill Street during the weekday evening peak hour operates at Level of Service (LOS) F with a 95th percentile queue of 138 feet, and a volume to capacity (v/c) ratio of 0.78. Based on our site visit, we confirmed the operations are consistent with the information presented in the memorandum. We note that during the 2031 No-Build and 2031 Build Conditions, the 95th percentile queue is anticipated to increase to 233 feet, and the v/c ratio is anticipated to increase to 1.09. We recommend VAI outline if there are any plans to implement mitigation at this intersection due to the operations at the Mill Street eastbound approach.

Response:

The impact of the Project at the Main Street/Mill Street intersection has been defined as an increase in average motorist delay of less than 1.0 seconds that resulted in a corresponding increase in vehicle queuing along the Mill Street approach of up to three (3) feet (less than one (1) vehicle). This level of impact does not warrant or require improvements to the intersection to accommodate the relatively minor increase in traffic that the Project represents (predicted to be an increase of one (1) vehicle during the peak hours).

Comment 23:

In Table 7, several of the measured sight distances are listed as "500+" feet. Based on our site visit, the intersection sight distance looking east from the Project Site Roadway along Gratuity Road is close to 500 feet and constrained by an existing horizontal curve. We recommend VAI provide a more specific measured sight distance to fully understand the available sight distance.

Response:

The measured sight distances presented in Table 7 of the April 2024 TIA (500+ feet) far exceed both the recommended minimum stopping sight distance (200 feet) and the desirable intersection sight distance (335 feet for left-turn movements) for Project site roadway intersections to function in a safe and efficient manner. Providing a more exacting measurement of a value in excess of 500 feet would not change this conclusion.

Comment 24:

Based on our site visit, the sight distances at the intersection of Main Street at Arlington Street are limited due to the crest vertical curve to the south on Main Street and the bridge abutment directly to the south of Arlington Street. Also, there is no marked crosswalk across Arlington Street despite the presence

⁶An extended approach grade of +5 percent or more can impact vehicle performance, which is not present at the study area intersections (*A Policy on Geometric Design of Highway and Streets*, 7th Edition; American Association of State Highway and Transportation Officials (AASHTO); Washington D.C.; 2018).

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of pedestrian ramps on both sides of Arlington Street on the west side of Main Street. We recommend VAI consider working with the Town to improve intersection sight distance and potential installation of a crosswalk at the Arlington Street eastbound approach to the intersection of Main Street.

Response:

An intersection ahead warning sign is provided on the Main Street northbound approach to Arlington Street to address the identified sight line limitation. To the extent so desired by the Town and subject to review and approval by MassDOT, the Project proponent will install a marked crosswalk across Arlington Street at Main Street.

Comment 25:

Based on our site visit at the intersection of Mill Street at Arlington Street and Gratuity Road, we note there are no yield signs, stop signs, or pavement markings to indicate proper traffic control for the northbound channelized right turn lane from Mill Street onto Arlington Street. We recommend VAI consider working with the Town to recommend proper signing and striping for the northbound channelized right turn lane from Mill Street onto Arlington Street.

Response:

To the extent so desired by the Town and subject to receipt of all necessary rights, permits and approvals, the Project proponent will design and constrict sign and pavement marking improvements for the channelized right-turn lane from Mill Street northbound to Arlington Street eastbound.

Comment 26:

Based on our site visit at the intersection of Mill Street and Jenkins Road, sight distance is limited due to the crest vertical curve to the south on Mill Street, and the Jenkins Road eastbound approach contains a splitter island to separate turns to/from the north and the south. Jenkins Road contains no centerline markings or signage and creates a potential conflict point where two streams of traffic merge at the west end of the island. Based on Figure 6 from the memorandum, Jenkins Road between Project Site Roadway (Therese Lane) and Mill Street is anticipated to service approximately 64% of Project-related traffic. We recommend VAI address how to better serve Project-related traffic along Jenkins Road, including investigating how to limit conflict points at the intersection of Mill Street and Jenkins Road, and how to improve sight distance south on Mill Street.

Response:

Discontinuing the southernmost channelized portion of Jenkins Road to create a T-intersection would serve to: i) reduce conflict points; and ii) improve sight lines by positioning the intersection further from the crest vertical curve along Mill Street to the south of Jenkins Road.

Comment 27:

The fourth item under Recommendations in the Project Access section on page 19 discusses conformance of the work with the Manual on Uniform Traffic Control Devices (MUTCD), and it references by footnote the 2009 edition of the MUTCD. By the time the Project is approved, the MUTCD 11th Edition may be adopted by MassDOT. Accordingly, we recommend VAI clarify that a reference to "the latest edition" of the MUTCD may be appropriate.

Response:

Any approvals that may be granted for the Project should indicate that the signs and pavement markings that are to be installed as a part of the Project



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shall comply with the latest edition of the MUTCD that has been approved by MassDOT.

Comment 28:

The plans indicate a proposed sidewalk on the east side of Project Site Roadway (Gerard Way) to connect to the south side of Gratuity Road, and the east side of Project Site Roadway (Therese Lane) to connect to the north side of Jenkins Road. We note the proposed sidewalks do not connect to existing or proposed sidewalks along either Gratuity Road or Jenkins Road. We recommend VAI consider working with the Town to facilitate how the proposed sidewalks may tie into plans for future pedestrian accommodations within the study area.

Response:

The Project proponent will coordinate with the Town to ensure that the proposed sidewalks along Gerard Way and Therese Lane are designed and constructed in a manner that will allow for connections to potential future sidewalks along Gratuity Road and Jenkins Road.

Comment 29:

The Project access along Jenkins Road is adjacent to the Fairgrounds and Hazel Grove Park. Given the potential for seasonal events at Fairground and/or Hazel Grove Park that could potentially result in additional vehicle trips and/or parking for these events along Jenkins Road, VAI should investigate how the Project would impact traffic operations and safety associated with potential events at Fairgrounds and Hazel Grove Park.

Response:

The Project will not result in a material impact on traffic operations or safety during events at the Fairgrounds or at Hazel Grove Park. As detailed in the April 2024 TIA and affirmed as a part of this response, the Project is expected to generate less than 13 vehicles during the weekday peak hours using conservative (high) traffic volume projections. With or without an event, the predicted traffic volume increase will not be readily apparent outside of the immediate proximity of the Project site.

Comment 30:

In our review of the June 5, 2023 TIA, we commented that if Transportation Demand Management (TDM) measures are required for the Project, the memorandum should include a section covering the proposed measures. VAI responded, in part, in their July 11, 2023, response letter that accompanied their updated TIA, "To the extent that there is a homeowners association, the association will make information available to new residents regarding available public transportation options in the area." Page 6 of the memorandum indicates "regularly scheduled public transportation services are not currently provided to or within the Town of Groton or in the immediate vicinity of the Project site." We recommend VAI include a discussion on TDM and additional measures beyond referencing public transportation that may not be immediately available.

Response:

In addition to providing public transportation schedules, the TDM program will include providing contact information for the Groton Council On Aging (COA) so that residents are informed of services that are offered by the COA, as well as information on bicycle facilities in the area, including the Nashua River Rail Trail which is located to the east of the Project site and can be accessed from Whistle Post Lane to the south (off of Pleasant Street). The intent of the TDM program is to inform residents of alternatives to the



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use of single-occupancy vehicles (SOVs) that are available in the area even though these services are not currently available at the Project site.

We trust that this information is responsive to the comments that were identified in the September 10, 2024 letter prepared by Nitsch concerning their review of the Project. If you should have any questions or would like to discuss our responses in more detail, please feel free to contact me.

Sincerely,

VANASSE & ASSOCIATES, INC.

PERFORM S. Dirk

Geffrey S. Dirk, F.E., PTOE, FITE

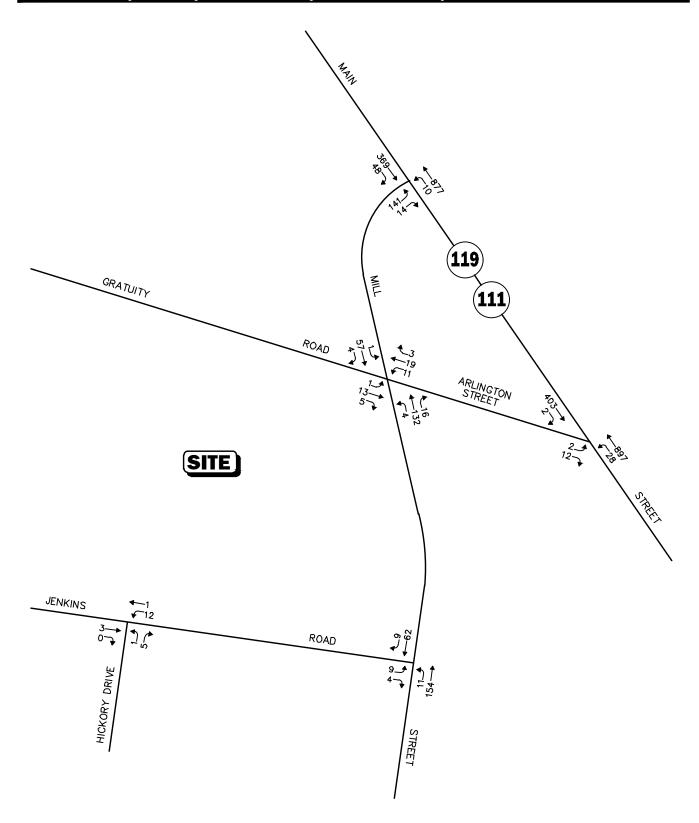
Managing Partner

Professional Engineer in CT, MA, ME, NH, RI, and VA

JSD/jsd

Attachments







Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

Not To Scale

Figure 5R



2031 No-Build Weekday Evening Peak-Hour Traffic Volumes

Table 6A UNSIGNALIZED INTERSECTION LEVEL-OF-SERVICE AND VEHICLE QUEUE SUMMARY

		2024 E	xisting			2031 No	-Build			2031 E	Build	
Unsignalized Intersection/Peak-Hour/Movement	V/C ^a	Delay ^b	LOS°	Queue ^d 95 th	V/C	Delay	LOS	Queue 95 th	V/C	Delay	LOS	Queue 95 th
Main Street at Mill Street												
Weekday Morning:												
Mill St. EB LT/RT	0.22	27.6	D	20	0.31	37.3	E	30	0.32	37.6	Е	33
Main St. NB LT	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0
Weekday Evening:												
Mill St. EB LT/RT	0.78	65.3	F	138	1.10	154.7	F	235	1.10	154.7	F	235
Main St. NB LT/TH	0.01	8.1	A	0	0.01	8.3	A	0	0.01	8.3	A	0
Main Street at Arlington Street												
Weekday Morning:												
Arlington St. EB LT/RT	0.18	20.3	C	15	0.23	24.5	C	23	0.27	25.4	D	25
Main St. NB LT	0.02	10.2	В	3	0.02	10.9	В	3	0.03	10.9	В	3
Weekday Evening:												
Arlington St. EB LT/RT	0.05	13.5	В	3	0.06	14.8	В	5	0.06	14.3	В	5
Main St. NB LT	0.03	8.1	A	3	0.03	8.3	A	3	0.03	8.3	A	3
Mill Street at Arlington Street and Gratuity Road												
Weekday Morning:												
Gratuity Rd. EB LT/TH/RT	0.04	8.2	A	3	0.05	8.3	A	5	0.06	8.3	A	5
Arlington St. WB LT/TH/RT	0.07	8.2	A	5	0.07	8.3	A	5	0.08	8.4	A	5
Mill St. NB LT/TH/RT	0.12	8.4	A	10	0.13	8.5	A	10	0.13	8.6	A	13
Mill St. SB LT/TH/RT	0.20	8.2	A	18	0.22	8.5	A	20	0.22	8.5	A	20
Weekday Evening:												
Gratuity Rd. EB LT/TH/RT	0.05	7.6	A	5	0.05	7.8	A	5	0.06	7.8	A	5
Arlington St. WB LT/TH/RT	0.09	8.0	A	8	0.10	8.1	A	8	0.11	8.3	A	10
Mill St. NB LT/TH/RT	0.22	8.3	A	20	0.25	8.6	A	25	0.26	8.7	A	25
Mill St. SB LT/TH/RT	0.08	7.7	A	5	0.09	7.8	A	8	0.09	7.9	A	8
Mill Street at Jenkins Road												
Weekday Morning:												
Jenkins Rd. EB LT/RT	0.04	9.9	A	3	0.04	10.0	В	3	0.05	10.1	В	5
Mill St. NB LT	0.00	7.6	A	0	0.00	7.6	A	0	0.00	7.6	A	0
Weekday Evening:												
Jenkins Rd. EB LT/RT	0.02	9.5	A	3	0.02	9.6	A	3	0.03	9.7	A	3
Mill St. NB LT	0.01	7.4	A	0	0.01	7.4	A	0	0.01	7.4	A	0

See notes at end of Table.

Table 6A (Continued) UNSIGNALIZED INTERSECTION LEVEL-OF-SERVICE AND VEHICLE QUEUE SUMMARY

		2024 E	xisting			2031 No	-Build			2031 E	Build	
Unsignalized Intersection/Peak-Hour/Movement	V/C ^a	Delay ^b	LOSc	Queue ^d 95 th	V/C	Delay	LOS	Queue 95 th	V/C	Delay	LOS	Queue 95 th
Jenkins Road at Hickory Drive												
Weekday Morning:												
Jenkins Rd. WB LT	0.01	7.2	A	0	0.01	7.2	A	0	0.01	7.2	Α	0
Hickory Dr. NB LT/RT	0.03	8.5	A	3	0.03	8.5	A	3	0.03	8.5	A	3
Weekday Evening:												
Jenkins Rd. WB LT	0.01	7.2	A	0	0.01	7.2	A	0	0.01	7.2	A	0
Hickory Dr. NB LT/RT	0.01	8.4	A	0	0.01	8.4	A	0	0.01	8.4	Α	0
Gratuity Road at the Project Site Roadway Weekday Morning:												
Gratuity Rd. WB LT									0.00	7.3	A	0
Project Site Roadway NB LT/RT									0.00	8.4	A	0
Weekday Evening:									0.00	0		Ŭ
Gratuity Rd. WB LT									0.00	7.3	A	0
Project Site Roadway NB LT/RT									0.00	8.4	A	0
Jenkins Road at the Project Site Roadway												
Weekday Morning:												
Jenkins Rd. EB LT									0.00	8.7	A	0
Project Site Roadway SB LT/RT									0.00	0.0	A	0
Weekday Evening:												
Jenkins Rd. EB LT									0.00	8.7	A	0
Project Site Roadway SB LT/RT									0.00	0.0	A	0

NB = northbound, EB = eastbound; SB = southbound; WB = westbound; LT = left-turning movements; TH = through movements; RT = right-turning movements.

^aVolume to Capacity ratio. ^bAverage control delay per vehicle (in seconds). ^cLevel of service.

^dQueue length in feet.

ATTACHMENTS

ITE PARKING GENERATION DATA SEASONAL ADJUSTMENT DATA AUTOMATIC TRAFFIC RECORDER DATA MASSDOT CRASH DATA WORKSHEETS ITE TRIP GENERATION DATA CAPACITY ANALYSIS WORKSHEETS



ITEParkGen Web App

How to Use ITEParkGen

PGM Desk Reference

Support Documents

Add Users

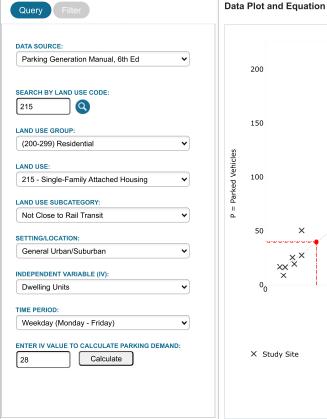
E Comments

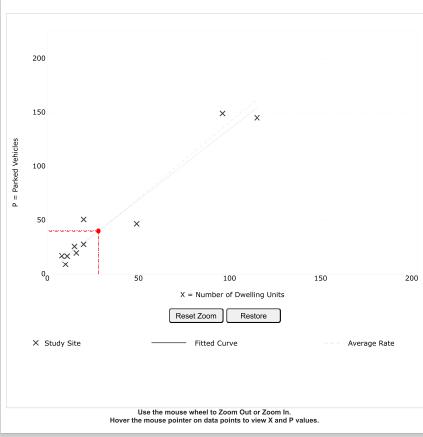


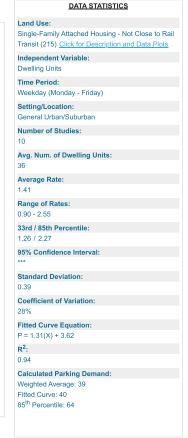














Massachusetts Highway Department Statewide Traffic Data Collection 2019 Weekday Seasonal Factors

Factor Group	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	ОСТ	NOV	DEC	Axle Factor
R1	1.22	1.14	1.12	1.06	1.00	0.96	0.87	0.85	0.96	0.99	1.04	1.12	0.85
R2	0.95	0.96	0.98	0.97	0.97	0.93	0.97	0.94	0.96	0.90	0.92	0.93	0.96
R3	1.15	1.06	1.07	1.00	0.89	0.88	0.89	0.89	0.95	0.92	1.02	1.01	0.97
R4-R7	1.09	1.09	1.11	1.02	0.96	0.92	0.89	0.89	0.99	0.98	1.09	1.13	0.98
U1-Boston	1.03	1.01	0.98	0.94	0.94	0.92	0.95	0.93	0.94	0.94	0.97	1.04	0.96
U1-Essex	1.09	1.06	1.03	0.99	0.94	0.90	0.88	0.86	0.93	0.94	0.99	1.06	0.93
U1-Southeast	1.06	1.05	1.01	0.97	0.95	0.93	0.93	0.90	0.94	0.94	0.98	1.04	0.98
U1-West	1.19	1.14	1.09	0.95	0.92	0.89	0.89	0.86	0.91	0.95	0.97	1.07	0.84
U1-Worcester	1.02	1.04	0.97	0.94	0.93	0.91	0.95	0.91	0.93	0.92	0.95	1.10	0.88
U2	1.01	1.00	0.94	0.93	0.91	0.89	0.93	0.90	0.90	0.91	0.94	1.02	0.99
U3	1.06	1.03	0.98	0.94	0.93	0.91	0.95	0.91	0.92	0.93	0.97	1.00	0.98
U4-U7	1.01	1.00	0.95	0.92	0.88	0.86	0.92	0.91	0.92	0.94	0.99	1.04	0.99
Rec - East	1.04	1.16	1.12	0.98	0.92	0.88	0.77	0.81	0.94	1.02	1.08	1.12	0.99
Rec - West	1.30	1.23	1.32	1.18	0.95	0.82	0.70	0.69	0.97	0.96	1.16	1.15	0.98

Round off:

0-999 = 10

>1000 = 100

U = Urban

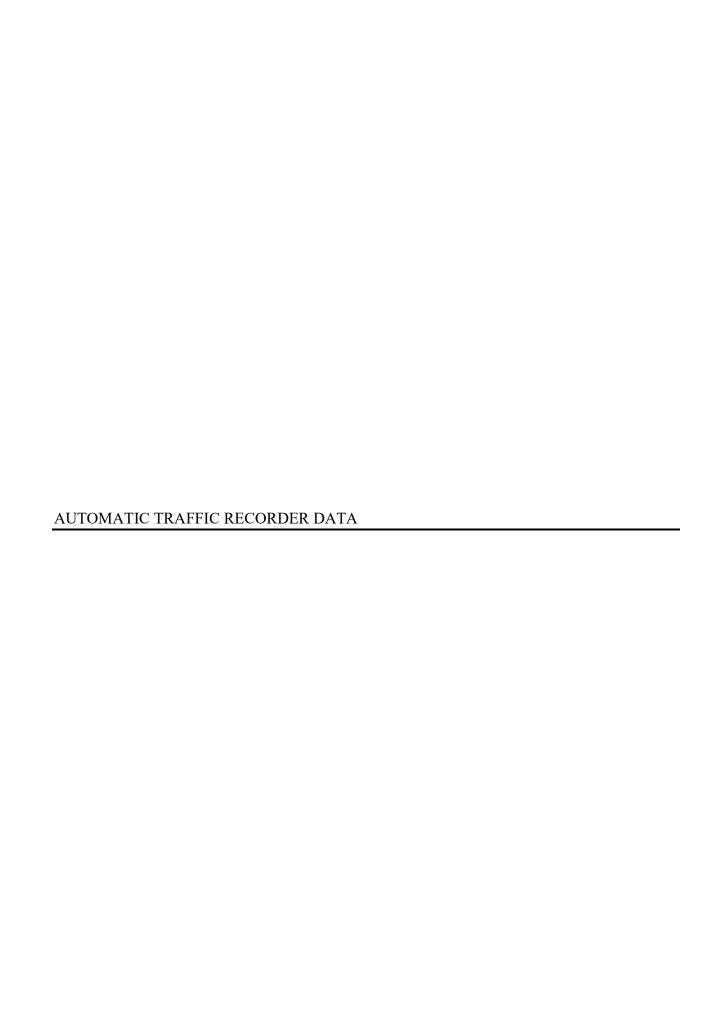
R = Rural

- 1 Interstate
- 2 Freeway and Expressway
- 3 Other Principal Arterial
- 4 Minor Arterial
- 5 Major Collector
- 6 Minor Collector
- 7 Local Road and Street

Recreational - East Group - Cape Cod (all towns) including the town of Plymouth south of Route 3A (stations 7014,7079,7080,7090,7091,7092,7093,7094,7095,7096,7097,7108 and 7178), Martha's Vineyard and Nantucket.

Recreational - West Group - Continuous Stations 2 and 189 including stations

1066,1067,1083,1084,1085,1086,1087,1088,1089,1090,1091,1092,1093,1094,1095,1096,1097,1098,1099,1100,1101,1102,1103,1104,1105,1106,1107,1108,1113,1114, 1116,2196,2197 and 2198.



Location: Gratuity Road Location: East of Mill Street City/State: Groton, MA Site Code: 81020001

3/6/2024	on, ivia EE	2	Hour ⁻	Totals	W		Hour 7	Totals	Combine	d Totals
Time	Morning	Afternoon	Morning	Afternon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon
12:00	0	0	Worring	Alternon	0	0	Morning	Aitemoon	Worming	Aitemoon
12:15	0	0			0	1				
12:30	0	1			0	0				
12:45	0	1	0	2	0	1	0	2	0	4
1:00	0	1	•	_	0	1		_		
1:15	0	0			0	0				
1:30	0	3			0	5				
1:45	0	1	0	5	0	1	0	7	0	12
2:00	0	0			0	1				
2:15	0	4			0	2				
2:30	0	0			0	3				
2:45	0	0	0	4	0	1	0	7	0	11
3:00	1	2			1	1				
3:15	0	0			0	2				
3:30	1	3			1	1				
3:45	0	3	2	8	0	4	2	8	4	16
4:00	0	1			0	5				
4:15	0	1			0	1				
4:30	1	2			0	2				
4:45	0	1	1	5	0	2	0	10	1	15
5:00	0	2			0	2				
5:15	0	1			0	2				
5:30	0	0			0	1				
5:45	0	1	0	4	0	1	0	6	0	10
6:00	1	1			0	2				
6:15	1	1			2	1				
6:30	2	0			0	2				
6:45	3	3	7	5	1	0	3	5	10	10
7:00	3	0			2	2				
7:15	1	2			0	1				
7:30	3	0			0	1				
7:45	0	3	7	5	2	1	4	5	11	10
8:00	2	0			0	0				
8:15	6	0			2	1				
8:30	0	0	_		0	0				
8:45	1	0	9	0	2	1	4	2	13	2
9:00	3	0			1	1				
9:15	1	2			1	2				
9:30 9:45	2 2	0	8	3	0	0	3	2	11	6
	0	0	0	3	1	0	3	3	1.1	0
10:00 10:15	1	0			1	0				
10:30	3				4					
10:45	1	0	5	0	0	0	6	0	11	0
11:00	2	1	5	0	1	0	U	U	- 11	U
11:15	1	0			1	0				
11:30	1	0			1	0				
11:45	0	0	4	1	2	1	5	1	9	2
Total	43	42		'	27	56	<u> </u>	'	70	98
Percent	50.6%	49.4%			32.5%	67.5%			41.7%	58.3%
. Groont	30.070	10.770			32.070	37.070			71.70	30.070

Location: Gratuity Road Location: East of Mill Street City/State: Groton, MA Site Code: 81020001

City/State: Grote			=							
3/7/2024	EB ·		Hour T		WE		Hour 7		Combined	
Time	Morning	Afternoon	Morning	Afternon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon
12:00	0	0			0	1				
12:15	1	1			0	1				
12:30	0	1		_	0	1	_	_		_
12:45	0	0	1	2	0	0	0	3	1	5
1:00	0	3			0	0				
1:15	0	0			0	1				
1:30	0	1	_	_	0	3	_	_	_	
1:45	0	2	0	6	0	3	0	7	0	13
2:00	0	2			0	2				
2:15	0	1			0	2				
2:30	0	0	•		0	2		_		
2:45	0	1	0	4	1	1	1	7	1	11
3:00	1	2			0	2				
3:15	0	2			0	2				
3:30	0	2		_	0	5	_			
3:45	0	1	1	7	0	1	0	10	1	17
4:00	0	1			0	3				
4:15	0	0			0	3				
4:30	0	0	_	_	0	2	_		_	
4:45	0	1	0	2	0	3	0	11	0	13
5:00	0	2			0	4				
5:15	0	3			0	1				
5:30	1	1	_	_	0	5	_		_	
5:45	1	0	2	6	0	3	0	13	2	19
6:00	2	3			1	1				
6:15	0	2			1	2				
6:30	2	0	_	_	0	2	_	_		
6:45	1	2	5	7	1	1	3	6	8	13
7:00	1	1			0	1				
7:15	1	1			2	0				
7:30	2	1		_	0	2		_		
7:45	2	2	6	5	2	2	4	5	10	10
8:00	2	0			0	1				
8:15	5	2			1	1				
8:30	3	1		_	1	0				_
8:45	1	0	11	3	1	2	3	4	14	7
9:00	0	1			0	0				
9:15	0	0			2	0				
9:30	0	0	•		1	0				
9:45	0	1	0	2	0	0	3	0	3	2
10:00	0	0			0	0				
10:15	2	0			0	0				
10:30	1	0			0	0	0	4		
10:45	0	0	3	0	3	1	3	1	6	1
11:00	1	0			0	0				
11:15	1	0			0	0				
11:30	1	0		0	1	0		0		•
11:45	11	0	4	0	3	0	4	0	8	0
Total	33	44			21	67			54	111
Percent	42.9%	57.1%			23.9%	76.1%			32.7%	67.3%
Grand Total	76	86			48	123			124	209
Percent	46.9%	53.1%			28.1%	71.9%			37.2%	62.8%
ADT		ADT: 166		AADT: 166						

Location: Gratuity Road Location: East of Mill Street City/State: Groton, MA Site Code: 81020001

	Mond		Tues	sday	Wednes		Thursd		Frida	ıy	Satur		Sund	lay	Week Ave	erage
Time	EB,	WB,	EB,	WB,	EB,	WB,	EB,	WB,	EB,	WB,	EB,	WB,	EB,	WB,	EB,	WB,
0:00	*	*	*	*	0	0	1	0	*	*	*	*	*	*	0	0
1:00	*	*	*	*	0	0	0	0	*	*	*	*	*	*	0	0
2:00	*	*	*	*	0	0	0	1	*	*	*	*	*	*	0	0
3:00	*	*	*	*	2	2	1	0	*	*	*	*	*	*	2	1
4:00	*	*	*	*	1	0	0	0	*	*	*	*	*	*	0	0
5:00	*	*	*	*	0	0	2	0	*	*	*	*	*	*	1	0
6:00	*	*	*	*	7	3	5	3	*	*	*	*	*	*	6	3
7:00	*	*	*	*	7	4	6	4	*	*	*	*	*	*	6	4
8:00	*	*	*	*	9	4	11	3	*	*	*	*	*	*	10	4
9:00	*	*	*	*	8	3	0	3	*	*	*	*	*	*	4	3
10:00	*	*	*	*	5	6	3	3	*	*	*	*	*	*	4	4
11:00	*	*	*	*	4	5	4	4	*	*	*	*	*	*	4	4
12:00	*	*	*	*	2	2	2	3	*	*	*	*	*	*	2	2
13:00	*	*	*	*	5	7	6	7	*	*	*	*	*	*	6	7
14:00	*	*	*	*	4	7	4	7	*	*	*	*	*	*	4	7
15:00	*	*	*	*	8	8	7	10	*	*	*	*	*	*	8	9
16:00	*	*	*	*	5	10	2	11	*	*	*	*	*	*	4	10
17:00	*	*	*	*	4	6	6	13	*	*	*	*	*	*	5	10
18:00	*	*	*	*	5	5	7	6	*	*	*	*	*	*	6	6
19:00	*	*	*	*	5	5	5	5	*	*	*	*	*	*	5	5
20:00	*	*	*	*	0	2	3	4	*	*	*	*	*	*	2	3
21:00	*	*	*	*	3	3	2	0	*	*	*	*	*	*	2	2
22:00	*	*	*	*	0	0	0	1	*	*	*	*	*	*	0	0
23:00	*	*	*	*	1	1	0	0	*	*	*	*	*	*	0	0
Total	0	0	0	0	85	83	77	88	0	0	0	0	0	0	81	84
Day	0		0)	168		165		0		0		0		165	
AM Peak					8:00	10:00	8:00	7:00							8:00	7:00
Volume					9	6	11	4							10	4
PM Peak					15:00	16:00	15:00	17:00							15:00	16:00
Volume					8	10	7	13							8	10
Comb Total	0		0	1	168		165		0		0		0		165	

1

Location: Jenkins Road Location: at # 80 City/State: Groton, MA Site Code: 81020002

City/State. Grot										
3/6/2024	E		Hour ⁻		W		Hour ⁻		Combine	
Time	Morning	Afternoon	Morning	Afternon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon
12:00	0	0			1	0				
12:15	0	0			0	1				
12:30	0	2			0	3				
12:45	0	0	0	2	0	2	1	6	1	8
1:00	0	2			0	2				
1:15	0	1			0	3				
1:30	0	0			0	1				
1:45	0	0	0	3	0	5	0	11	0	14
2:00	0	2			0	4				
2:15	0	0			0	3				
2:30	0	2			0	3				
2:45	0	0	0	4	0	3	0	13	0	17
3:00	0	4	· ·	7	0	1	o o	10	J	.,
3:15	0	2			0	3				
3:30	0	2			0	5				
3:45	0	3	0	11	0	2	0	11	0	22
4:00	0	2	U	11		5	U	11	U	22
					0					
4:15	0	0			0	3				
4:30	0	1			0	5				
4:45	0	1	0	4	0	1	0	14	0	18
5:00	0	0			0	2				
5:15	0	2			0	4				
5:30	0	1			0	3				
5:45	1	1	1	4	0	4	0	13	1	17
6:00	1	1			0	1				
6:15	0	1			0	3				
6:30	2	1			1	0				
6:45	1	0	4	3	2	0	3	4	7	7
7:00	1	1			0	2				
7:15	0	0			2	4				
7:30	1	1			0	2				
7:45	2	0	4	2	0	0	2	8	6	10
8:00	0	1			2	3				
8:15	3	0			4	2				
8:30	6	0			2	3				
8:45	0	1	9	2	7	1	15	9	24	11
9:00	3	0	-	_	3	2		_		
9:15	1	0			1	0				
9:30	0	0			1	0				
9:45	0	0	4	0	1	0	6	2	10	2
10:00	2	0	-	0	1	0	0	2	10	2
10:00	1	0			1	0				
10:13	0	0			0	0				
10:30	0	0	3	0	4	0	6	0	9	0
			3	Ü			О	U	9	U
11:00	0	0			1	0				
11:15	1	0			3	1				
11:30	2	0	•	_	0	1	-	_	_	
11:45	0	0	3	0	2	0	6	2	9	2
Total	28	35			39	93			67	128
Percent	44.4%	55.6%			29.5%	70.5%			34.4%	65.6%

Location: Jenkins Road Location: at # 80 City/State: Groton, MA Site Code: 81020002

City/State: Grott										
3/7/2024	EB		Hour To		WE		Hour 1		Combined	
Time		Afternoon	Morning	Afternon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon
12:00	0	0			0	0				
12:15	0	2			0	2				
12:30	0	0		_	0	1			_	
12:45	0	3	0	5	0	1	0	4	0	9
1:00	0	1			0	5				
1:15	0	3			0	4				
1:30	0	0			0	1				
1:45	0	0	0	4	0	2	0	12	0	16
2:00	0	1			0	4				
2:15	0	0			0	2				
2:30	0	1	_		0	2	_		_	
2:45	0	2	0	4	0	0	0	8	0	12
3:00	0	1			0	2				
3:15	0	1			0	3				
3:30	0	0			0	5				
3:45	1	0	1	2	0	5	0	15	1	17
4:00	0	2			0	4				
4:15	0	0			0	5				
4:30	0	2			0	1				
4:45	0	2	0	6	0	5	0	15	0	21
5:00	0	0			0	2				
5:15	0	1			0	2				
5:30	1	2 2		_	0	2				
5:45	0		1	5	0	2	0	8	1	13
6:00	1	0			0	2				
6:15	1	3			0	1				
6:30	1	2		_	2	3	_			
6:45	1	2	4	7	0	4	2	10	6	17
7:00	0	0			2	2				
7:15	0	0			0	0				
7:30	1	0	_		0	2	_	_	_	_
7:45	2	0	3	0	3	1	5	5	8	5
8:00	3	0			3	1				
8:15	0	0			2	3				
8:30	3	1			0	1	_	_		_
8:45	0	0	6	1	4	2	9	7	15	8
9:00	0	0			1	1				
9:15	1	0			2	3				
9:30	2	0			1	1		_	4.0	_
9:45	1	0	4	0	2	0	6	5	10	5
10:00	0	1			0	0				
10:15	1	0			0	1				
10:30	3	0	_		1	0		4	•	
10:45	3	0	7	1	1	0	2	1	9	2
11:00	1	0			0	1				
11:15	1	0			1	2				
11:30	2	0	_		1	0			_	
11:45	1	0	5	0	0	0	2	3	7	3
Total	31	35			26	93			57	128
Percent	47.0%	53.0%			21.8%	78.2%			30.8%	69.2%
Grand Total	59	70 54.20/			65 25 0%	186			124	256
Percent	45.7%	54.3%			25.9%	74.1%			32.6%	67.4%
ADT		ADT: 190		AADT: 190		l		I		

Location: Jenkins Road Site Code: 81020002

Location: at #80 City/State: Groton, MA

otate. Groto	Mond		Tueso	day	Wednes		Thursda		Frida		Satur		Sunda		Week Ave	rage
Time	EB,	WB,	EB,	WB,	EB,	WB,	EB,	WB,	EB,	WB,	EB,	WB,	EB,	WB,	EB,	WB,
0:00	*	*	*	*	0	1	0	0	*	*	*	*	*	*	0	
1:00	*	*	*	*	0	0	0	0	*	*	*	*	*	*	0	
2:00	*	*	*	*	0	0	0	0	*	*	*	*	*	*	0	
3:00	*	*	*	*	0	0	1	0	*	*	*	*	*	*	0	
4:00	*	*	*	*	0	0	0	0	*	*	*	*	*	*	0	
5:00	*	*	*	*	1	0	1	0	*	*	*	*	*	*	1	
6:00	*	*	*	*	4	3	4	2	*	*	*	*	*	*	4	
7:00	*	*	*	*	4	2	3	5	*	*	*	*	*	*	4	
8:00	*	*	*	*	9	15	6	9	*	*	*	*	*	*	8	
9:00	*	*	*	*	4	6	4	6	*	*	*	*	*	*	4	
10:00	*	*	*	*	3	6	7	2	*	*	*	*	*	*	5	
11:00	*	*	*	*	3	6	5	2	*	*	*	*	*	*	4	
12:00	*	*	*	*	2	6	5	4	*	*	*	*	*	*	4	
13:00	*	*	*	*	3	11	4	12	*	*	*	*	*	*	4	
14:00	*	*	*	*	4	13	4	8	*	*	*	*	*	*	4	
15:00	*	*	*	*	11	11	2	15	*	*	*	*	*	*	6	
16:00	*	*	*	*	4	14	6	15	*	*	*	*	*	*	5	
17:00	*	*	*	*	4	13	5	8	*	*	*	*	*	*	4	
18:00	*	*	*	*	3	4	7	10	*	*	*	*	*	*	5	
19:00	*	*	*	*	2	8	0	5	*	*	*	*	*	*	1	
20:00	*	*	*	*	2	9	1	7	*	*	*	*	*	*	2	
21:00	*	*	*	*	0	2	0	5	*	*	*	*	*	*	0	
22:00	*	*	*	*	0	0	1	1	*	*	*	*	*	*	0	
23:00	*	*	*	*	0	2	0	3	*	*	*	*	*	*	0	
Total	0	0	0	0	63	132	66	119	0	0	0	0	0	0	65	1
Day	0		0		195		185		0		0		0		188	
AM Peak					8:00	8:00	10:00	8:00							8:00	8
Volume					9	15	7	9							8	
PM Peak					15:00	16:00	18:00	15:00							15:00	16
Volume					11	14	7	15							6	
mb Total	0		0		195		185		0		0		0		188	

1





INTERSECTION CRASH RATE WORKSHEET

CITY/TOWN : Groton				COUNT DA	TE:	5/13/2019
DISTRICT: 3	UNSIGN	ALIZED :	Х	SIGNA	LIZED :	
		~ IN	TERSECTION	I DATA ~		
MAJOR STREET :	Main Street					
MINOR STREET(S):	Mill Street					
INTERSECTION DIAGRAM (Label Approaches)	↑ North			MetgsElane Massage Thetapy	Tag 3	
APPROACH :	1	2	PEAK HOUF	R VOLUMES 4	5	Total Peak
DIRECTION:	NB	SB	EB	WB	•	Hourly Approach
PEAK HOURLY VOLUMES (PM) :	787	360	140			1,287
"K" FACTOR:	0.090	INTERS	ECTION ADT APPROACH		AL DAILY	14,300
TOTAL # OF CRASHES :	8	# OF YEARS :	5	CRASHES	GE # OF PER YEAR (.):	1.60
CRASH RATE CALCU	LATION :	0.31	RATE =	(A * 1,0	000,000) * 365)	
Comments : Below Mas	sDOT Statew	ide and Distri	ct Average Cı	rash Rates		



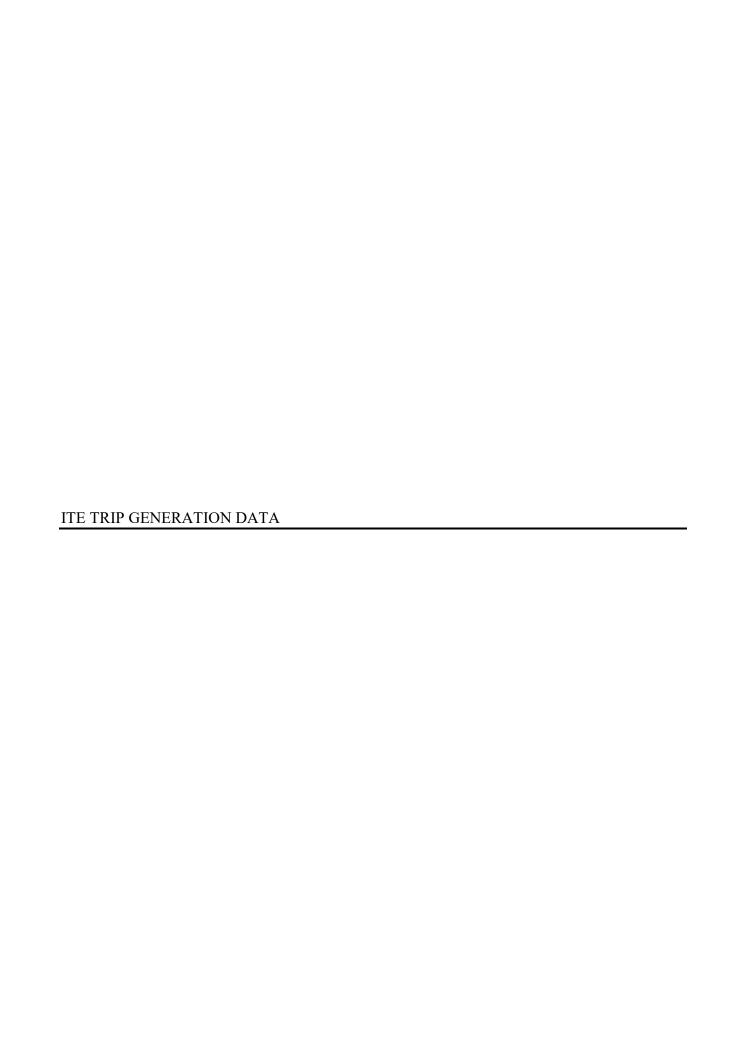
INTERSECTION CRASH RATE WORKSHEET

CITY/TOWN : Groton				COUNT DA	TE:	5/13/2019
DISTRICT: 3	UNSIGN	ALIZED :	Х	SIGNA	LIZED :	
		~ IN	TERSECTION	I DATA ~		
MAJOR STREET :	Mill Street					_
MINOR STREET(S):	Arlington Stre	eet				
	Gratuity Road	d				
INTERSECTION DIAGRAM (Label Approaches)	↑ North	nsa /	agust na	Perhaps Reg. Anthropism St. Marie S	Samily Dog Training	Arings
APPROACH :	1	2	PEAK HOUF	4	5	Total Peak Hourly
DIRECTION :	NB	SB	EB	WB		Approach Volume
PEAK HOURLY VOLUMES (PM):	137	54	18	31		240
"K" FACTOR:	0.090	INTERS	ECTION ADT APPROACH	2,667		
TOTAL # OF CRASHES :	1	# OF YEARS :	5	AVERAGE # OF CRASHES PER YEAR (0.20 A):		0.20
CRASH RATE CALCULATION :		0.21	RATE = (A * 1,000,000) (V * 365)			
Comments : Below Mas						



INTERSECTION CRASH RATE WORKSHEET

CITY/TOWN : Groton				COUNT DA	TE:	5/13/2019
DISTRICT: 3	UNSIGN	IALIZED :	Х	SIGNA	LIZED :	
		~ IN	TERSECTION	I DATA ~		
MAJOR STREET :	Jenkins Roa	d				
MINOR STREET(S):	Hickory Drive	е				
INTERSECTION DIAGRAM	↑ North	Fairgrounds h	ď			
(Label Approaches)				aligrounds Ru	Frounds Rg Jenkins Rg	Fairgrounds Rd
			PEAK HOUF	San	Founds Rg Jenkins Rg	Fairgrounds Rd Total Peak
(Label Approaches) APPROACH:	1	2	PEAK HOUF	San	Frounds Rd Jenkins Rd	Total Peak Hourly
	1 NB	2 SB		R VOLUMES	→enkins Ril	
APPROACH :			3	R VOLUMES 4	→enkins Ril	Hourly Approach
APPROACH: DIRECTION: PEAK HOURLY	NB	SB	3 EB	R VOLUMES 4 WB 10 (V) = TOTA	55	Hourly Approach Volume
APPROACH: DIRECTION: PEAK HOURLY VOLUMES (AM):	NB 22	SB	3 EB 2 ECTION ADT	WB 10 (V) = TOTA VOLUME: AVERA CRASHES	55	Hourly Approach Volume 34
APPROACH: DIRECTION: PEAK HOURLY VOLUMES (AM): "K" FACTOR:	NB 22 0.090	SB INTERSI # OF	3 EB 2 ECTION ADT APPROACH	WB 10 (V) = TOTA VOLUME: AVERA CRASHES A	5 AL DAILY GE # OF PER YEAR ():	Hourly Approach Volume 34 378









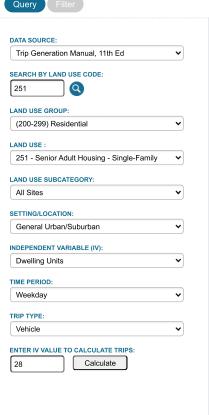


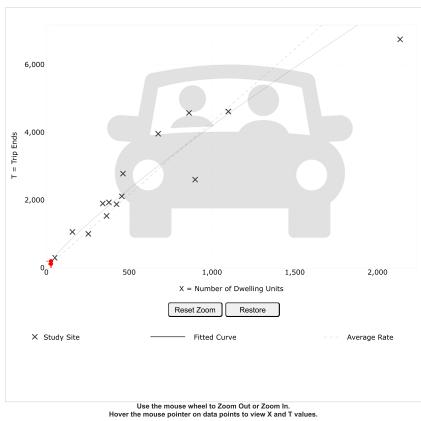
Graph Look Up

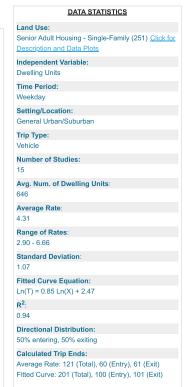
Data Plot and Equation













Add-ons to do more





ITETripGen Web-based App

Graph Look Up

How to Use ITETripGen

TGM Desk Reference

Support Documents

Add Users

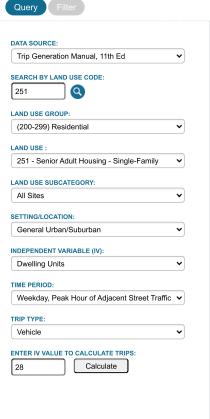
E Comments

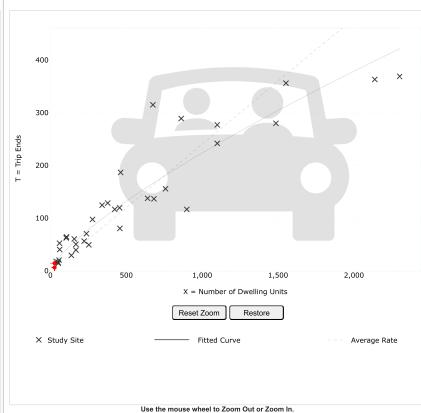






Graph Look Up





Hover the mouse pointer on data points to view X and T values.

Data Plot and Equation



R²: 0.88 **Directional Distribution:** 33% entering, 67% exiting

Calculated Trip Ends: Average Rate: 7 (Total), 2 (Entry), 5 (Exit)

0.24 Range of Rates

0.10

0.13 - 0.84

Standard Deviation:

Fitted Curve Equation:

Ln(T) = 0.76 Ln(X) + 0.16

Fitted Curve: 15 (Total), 5 (Entry), 10 (Exit)

Add-ons to do more

Try OTISS Pro



Data Plot and Equation





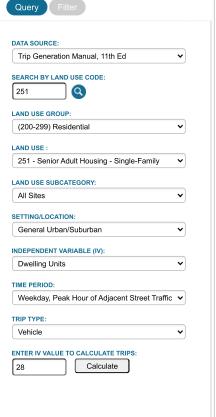


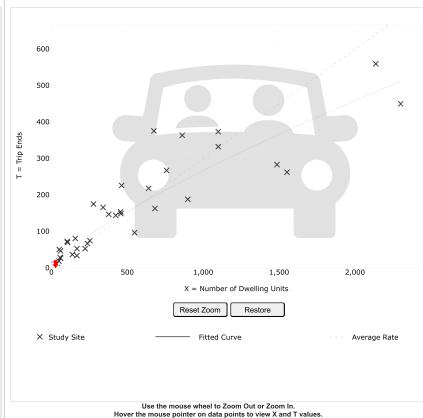


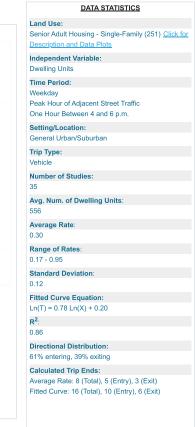




E Comments













Intersection						
Int Delay, s/veh	0.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	₩.	LDK	INDL	IND I) 	אמט
Traffic Vol, veh/h	36	5	0	173	805	116
Future Vol, veh/h	36	5	0	173	805	116
Conflicting Peds, #/hr	4	4	4	0	000	4
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -	None	-		-	None
Storage Length	0	NOHE -	-	None	-	None
Veh in Median Storage,		_	-	0	0	-
Grade, %	, # 0	-	-	0	0	_
Peak Hour Factor	93	93	82	82	85	85
				5		
Heavy Vehicles, %	0	0	0		1	0
Mvmt Flow	39	5	0	211	947	136
Major/Minor N	/linor2	N	//ajor1	N	//ajor2	
Conflicting Flow All	1234	1023	1087	0	-	0
Stage 1	1019	-	-	-	-	-
Stage 2	215	_	-	_	_	-
Critical Hdwy	6.4	6.2	4.1	-	_	-
Critical Hdwy Stg 1	5.4	-	-	_	_	_
Critical Hdwy Stg 2	5.4	_	_	_	_	_
Follow-up Hdwy	3.5	3.3	2.2	_	_	_
Pot Cap-1 Maneuver	197	289	649	_	_	_
Stage 1	351	-	-	_	_	_
Stage 2	826	_	_	_	_	_
Platoon blocked, %	020			_	_	_
Mov Cap-1 Maneuver	195	286	646	_	_	_
Mov Cap-1 Maneuver	195	200	-	_	_	_
	349	_	-	-		-
Stage 1	822			-		-
Stage 2	022	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s/v	27.6		0		0	
HCM LOS	D					
Minor Lane/Major Mvm	t	NBL	NRT	EBLn1	SBT	SBR
			ווטוו			ODIX
Capacity (veh/h) HCM Lane V/C Ratio		646		203 0.217	-	_
HCM Control Delay (s/v	(ob)	-		27.6	-	-
HOW CONTROL DEIAY (S/V	/ C II)	0	-		-	-
		٨				
HCM Lane LOS HCM 95th %tile Q (veh)	١ -	A 0	-	D 0.8	-	-

Intersection						
Int Delay, s/veh	0.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	\$	
Traffic Vol, veh/h	0	26	11	194	798	2
Future Vol, veh/h	0	26	11	194	798	2
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- Olop	None	-		-	None
Storage Length	0	-		-	_	-
Veh in Median Storage,		-	_	0	0	_
Grade, %	, # 0	_		0	0	_
Peak Hour Factor	50	50	81	81	80	80
Heavy Vehicles, %	0	8	0	4	2	0
		52	14		998	
Mvmt Flow	0	52	14	240	998	3
Major/Minor N	/linor2	N	Major1	N	/lajor2	
Conflicting Flow All	1268		1001	0		0
Stage 1	1000	_	-	-	-	_
Stage 2	268	_	_	_	_	_
Critical Hdwy	6.4	6.28	4.1	_	_	_
Critical Hdwy Stg 1	5.4	-	-	_	_	_
Critical Hdwy Stg 2	5.4	_	_	_	_	_
Follow-up Hdwy	3.5	3.372	2.2	_	_	_
Pot Cap-1 Maneuver	188	287	700	_	_	_
Stage 1	359		-	_	<u>-</u>	_
Stage 2	782	_	_	_	_	_
Platoon blocked, %	102			_	_	_
Mov Cap-1 Maneuver	184	287	700	_	_	
Mov Cap-1 Maneuver	184	201	700	_	_	_
	351	-	-	-		<u>-</u>
Stage 1				-		-
Stage 2	782	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s/v			NB 0.5		SB 0	
	20.3					
HCM Control Delay, s/v HCM LOS	20.3 C	MDI	0.5		0	ODD
HCM Control Delay, s/v HCM LOS Minor Lane/Major Mvmt	20.3 C	NBL	0.5	EBLn1		SBR
HCM Control Delay, s/v HCM LOS Minor Lane/Major Mvmt Capacity (veh/h)	20.3 C	700	0.5 NBT	287	0	SBR -
HCM Control Delay, s/v HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	20.3 C	700 0.019	0.5 NBT	287 0.181	0 SBT	SBR -
HCM Control Delay, s/v HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s/v	20.3 C	700 0.019 10.2	0.5 NBT 0	287 0.181 20.3	0 SBT	-
HCM Control Delay, s/v HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	/ 20.3 C	700 0.019	0.5 NBT	287 0.181	SBT	-

Intersection	
Intersection Delay, s/veh	/veh 8.3
Intersection LOS	A

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	3	9	7	17	1	2	3	65	11	3	119	2
Future Vol, veh/h	3	9	7	17	1	2	3	65	11	3	119	2
Peak Hour Factor	0.59	0.59	0.59	0.39	0.39	0.39	0.90	0.90	0.90	0.74	0.74	0.74
Heavy Vehicles, %	33	0	0	12	0	0	33	0	18	0	0	0
Mvmt Flow	5	15	12	44	3	5	3	72	12	4	161	3
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay, s/veh	8.2			8.2			8.4			8.2		
HCM LOS	Α			Α			Α			Α		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	4%	16%	85%	2%
Vol Thru, %	82%	47%	5%	96%
Vol Right, %	14%	37%	10%	2%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	79	19	20	124
LT Vol	3	3	17	3
Through Vol	65	9	1	119
RT Vol	11	7	2	2
Lane Flow Rate	88	32	51	168
Geometry Grp	1	1	1	1
Degree of Util (X)	0.116	0.044	0.069	0.196
Departure Headway (Hd)	4.769	4.923	4.841	4.217
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	755	729	742	856
Service Time	2.78	2.938	2.855	2.217
HCM Lane V/C Ratio	0.117	0.044	0.069	0.196
HCM Control Delay, s/veh	8.4	8.2	8.2	8.2
HCM Lane LOS	Α	Α	Α	Α
HCM 95th-tile Q	0.4	0.1	0.2	0.7

Intersection						
Int Delay, s/veh	1					
	EDI	EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y	0	0	₽	\$	7
Traffic Vol, veh/h	13	9	2	63	140	7
Future Vol, veh/h	13	9	2	63	140	7
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	-	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	79	79	86	86	82	82
Heavy Vehicles, %	15	0	0	2	1	0
Mvmt Flow	16	11	2	73	171	9
NA - ' /NA'	M O		1.1.1		4.1.0	
	Minor2		Major1		Major2	
Conflicting Flow All	253	176	180	0	-	0
Stage 1	176	-	-	-	-	-
Stage 2	77	-	-	-	-	-
Critical Hdwy	6.55	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.55	-	-	-	-	-
Critical Hdwy Stg 2	5.55	-	-	-	-	-
Follow-up Hdwy	3.635	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	708	872	1408	-	-	-
Stage 1	824	-	-	-	-	-
Stage 2	914	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	707	872	1408	-	-	-
Mov Cap-2 Maneuver	707	-	-	_	_	-
Stage 1	823	_	_	_	_	_
Stage 2	914	_	_	_	_	_
Clago 2	011					
Approach	EB		NB		SB	
HCM Control Delay, s/v	v 9.9		0.2		0	
HCM LOS	Α					
Minor Lane/Major Mvm	nt	NBL	NRT	EBLn1	SBT	SBR
	it.	1408		766		ODIX
Capacity (veh/h)		0.002	-	0.036	-	•
HCM Land V//C Datio					-	-
HCM Control Dolay (a)	(ab)	7.0		0.0		
HCM Control Delay (s/	veh)	7.6	0	9.9	-	-
	,	7.6 A 0	0 A	9.9 A 0.1	- -	-

Intersection						
Int Delay, s/veh	6.9					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u>₽</u>	בטוע	TIDE	4	¥	וטוז
Traffic Vol, veh/h	2	0	8	2	0	22
Future Vol, veh/h	2	0	8	2	0	22
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	_	-	_	-	0	-
Veh in Median Storage,	# 0	_	_	0	0	_
Grade, %	0	_	_	0	0	_
Peak Hour Factor	50	50	50	50	69	69
Heavy Vehicles, %	0	0	0	0	0	9
Mvmt Flow	4	0	16	4	0	32
WWW.CT IOW	•	J	10	•		UL.
		_		_		
	lajor1		Major2		/linor1	
Conflicting Flow All	0	0	4	0	40	4
Stage 1	-	-	-	-	4	-
Stage 2	-	-	-	-	36	-
Critical Hdwy	-	-	4.1	-	6.4	6.29
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.381
Pot Cap-1 Maneuver	-	-	1631	-	977	1059
Stage 1	-	-	-	-	1024	-
Stage 2	-	-	-	-	992	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1631	-	967	1059
Mov Cap-2 Maneuver	-	-	-	-	967	-
Stage 1	-	-	-	-	1024	-
Stage 2	-	-	-	_	982	-
3 3 9						
Annanach	ED		\A/D		NID	
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		5.8		8.5	
HCM LOS					Α	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		1059	-	-	1631	-
HCM Lane V/C Ratio		0.03	-	_	0.01	-
HCM Control Delay (s/v	eh)	8.5	-	-	7.2	0
HCM Lane LOS	,	Α	-	-	Α	A
HCM 95th %tile Q (veh)		0.1	-	-	0	-

Traffic Vol, veh/h 12 Future Vol, veh/h 12 Conflicting Peds, #/hr Sign Control Sto RT Channelized Storage Length Veh in Median Storage, # Grade, % Peak Hour Factor 8	BL E Y 1 27 1 0 0 0 0 0 49 r2 24 75 49 .4 .4	376 - - 6.2 -	9 9 1 Free 94 0 10 Major1 397 - 4.1 -	0 0 94 1 828	SBT 319 319 0 Free - 0 0 91 2 351 Major2	SBR 41 41 1 Free None 91 0 45
Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Future Vol, veh/h Sign Control RT Channelized Storage Length Veh in Median Storage, # Grade, % Peak Hour Factor Heavy Vehicles, % Mvmt Flow Major/Minor Minor Conflicting Flow All Stage 1 Stage 2 84 Critical Hdwy Stg 2 Follow-up Hdwy Stage 1 Stage 1 Stage 1 Stage 1 Stage 1 Gritical Hdwy Stg 2 Follow-up Hdwy Stg 2 Follow-up Hdwy Stage 1 Stage 2 Stage 1 Stage 2 Follow-up Hdwy Stage 1 Stage 1 Stage 2 Follow-up Hdwy Stage 1 Stage 1 Stage 2 Follow-up Hdwy Stage 1	227 227 227 1 1 200 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	13 13 1 Stop None - - - 85 0 15	9 9 1 Free - - - 94 0 10 Major1 - - 4.1	778 778 0 Free None - 0 0 94 1 828	319 319 0 Free - 0 0 91 2 351 Major2 - -	41 41 1 Free None - - - 91 0 45
Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Future Vol, veh/h Conflicting Peds, #/hr Sign Control RT Channelized Storage Length Veh in Median Storage, # Grade, % Peak Hour Factor Heavy Vehicles, % Mvmt Flow Major/Minor Minor Conflicting Flow All Stage 1 Stage 2 84 Critical Hdwy Stg 2 Follow-up Hdwy 3. Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-2 Maneuver Stage 1 Stage 1 Stage 1 Stage 1 Stage 2 Platoon blocked, % Mov Cap-2 Maneuver Stage 1 Stage 1 Stage 1 Stage 1 Stage 1 Stage 1 Stage 2 Platoon blocked, % Mov Cap-2 Maneuver Stage 1	227 227 227 1 1 200 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	13 13 1 Stop None - - - 85 0 15	9 9 1 Free - - - 94 0 10 Major1 - - 4.1	778 778 0 Free None - 0 0 94 1 828	319 319 0 Free - 0 0 91 2 351 Major2 - -	41 41 1 Free None - - - 91 0 45
Traffic Vol, veh/h 12 Future Vol, veh/h 12 Conflicting Peds, #/hr Sign Control Sto RT Channelized Storage Length Veh in Median Storage, # Grade, % Peak Hour Factor 8 Heavy Vehicles, % Mvmt Flow 14 Major/Minor Minor Conflicting Flow All 122 Stage 1 37 Stage 2 84 Critical Hdwy Stg 1 5. Critical Hdwy Stg 1 5. Critical Hdwy Stg 2 5. Follow-up Hdwy 3. Pot Cap-1 Maneuver 20 Stage 1 69 Stage 2 42 Platoon blocked, % Mov Cap-2 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68	227 227 1 1 pp § - N 0 0 0 0 0 0 0 85 0 49	13 1 Stop None - - - 85 0 15 M 376 - - - - 6.2	9 1 Free - - 94 0 10 Major1 - - 4.1	778 778 0 Free None - 0 0 94 1 828	319 319 0 Free - 0 0 91 2 351 Major2 - -	41 1 Free None 91 0 45
Future Vol, veh/h Conflicting Peds, #/hr Sign Control RT Channelized Storage Length Veh in Median Storage, # Grade, % Peak Hour Factor Heavy Vehicles, % Mvmt Flow Major/Minor Minor Conflicting Flow All Stage 1 Stage 2 84 Critical Hdwy Stg 1 Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-2 Maneuver Stage 1 Stage 1 Mov Cap-2 Maneuver Stage 1 Stage 1 Mov Cap-2 Maneuver Stage 1 Stage 1 Mov Cap-2 Maneuver Stage 1 Mov Cap-1 Maneuver Stage 1 Mov Cap-1 Maneuver Stage 1 Mov Cap-2 Maneuver Stage 1 Mov Cap-2 Maneuver Stage 1	227 1 pp S 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	13 1 Stop None - - - 85 0 15 M 376 - - - - 6.2	9 1 Free - - 94 0 10 Major1 - - 4.1	778 0 Free None - 0 0 94 1 828	319 0 Free - 0 0 91 2 351 Major2 - -	41 1 Free None 91 0 45
Conflicting Peds, #/hr Sign Control Sto RT Channelized Storage Length Veh in Median Storage, # Grade, % Peak Hour Factor 8 Heavy Vehicles, % Mvmt Flow 14 Major/Minor Minor Conflicting Flow All 122 Stage 1 37 Stage 2 84 Critical Hdwy Stg 1 5. Critical Hdwy Stg 1 5. Critical Hdwy Stg 2 5. Follow-up Hdwy 3. Pot Cap-1 Maneuver Stage 1 69 Stage 2 42 Platoon blocked, % Mov Cap-1 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68	1	1 Stop None - - - 85 0 15 - - - - - - - - - - - - - - - - - -	1 Free - - 94 0 10 Major1 - - 4.1	0 Free None - 0 0 94 1 828	0 Free - 0 0 91 2 351 Major2 - - -	1 Free None - - - 91 0 45
Sign Control Sto RT Channelized Storage Length Veh in Median Storage, # Grade, % Peak Hour Factor 8 Heavy Vehicles, % Mvmt Flow 14 Major/Minor Minor Conflicting Flow All 122 Stage 1 37 Stage 2 84 Critical Hdwy 6. Critical Hdwy Stg 1 5. Critical Hdwy Stg 2 5. Follow-up Hdwy 3. Pot Cap-1 Maneuver Stage 1 69 Stage 2 42 Platoon blocked, % Mov Cap-1 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68	- N 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Stop None 	Free 94 0 10 Major1 4.1	Free None - 0 0 0 94 1 828 N 0	Free 0 0 91 2 351 Major2	Free None 91 0 45
RT Channelized Storage Length Veh in Median Storage, # Grade, % Peak Hour Factor 8 Heavy Vehicles, % Mvmt Flow 14 Major/Minor Minor Conflicting Flow All 122 Stage 1 37 Stage 2 84 Critical Hdwy 66. Critical Hdwy Stg 1 5. Critical Hdwy Stg 2 5. Follow-up Hdwy 3. Pot Cap-1 Maneuver 20 Stage 1 69 Stage 2 42 Platoon blocked, % Mov Cap-1 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68	- N 0 0 0 0 0 0 855 0 0 449	None 85 0 15 None None	- - 94 0 10 Major1 397 - - 4.1	None	- 0 0 91 2 351 Major2 - -	None 91 0 45
Storage Length Veh in Median Storage, # Grade, % Peak Hour Factor Heavy Vehicles, % Mvmt Flow Major/Minor Conflicting Flow All Stage 1 Stage 2 Critical Hdwy 6. Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy 3. Pot Cap-1 Maneuver Stage 2 Platoon blocked, % Mov Cap-1 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68	0 0 0 0 85 0 49 rr2 24 775 49 .4 .4	85 0 15 N 376 - - 6.2	- 94 0 10 Major1 397 - 4.1	0 0 94 1 828	0 0 91 2 351 Major2 - -	91 0 45
Veh in Median Storage, # Grade, % Peak Hour Factor 8 Heavy Vehicles, % Mvmt Flow 14 Major/Minor Minor Conflicting Flow All 122 Stage 1 37 Stage 2 84 Critical Hdwy 6. Critical Hdwy Stg 1 5. Critical Hdwy Stg 2 5. Follow-up Hdwy 3. Pot Cap-1 Maneuver Stage 1 69 Stage 2 42 Platoon blocked, % Mov Cap-1 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68	0 0 35 0 49 r2 24 75 49 .4	85 0 15 N 376 - - 6.2	94 0 10 Major1 397 - 4.1	0 0 94 1 828	0 0 91 2 351 Major2 - - -	- - 91 0 45
Grade, % Peak Hour Factor Heavy Vehicles, % Mvmt Flow Major/Minor Conflicting Flow All Stage 1 Stage 2 84 Critical Hdwy 6. Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Stage 1 Mov Cap-2 Maneuver Stage 1 Stage 1 68	0 35 0 49 r2 24 75 49 .4 .4	85 0 15 N 376 - - 6.2	94 0 10 Major1 397 - 4.1	0 94 1 828 0 -	0 91 2 351 Major2 - - -	91 0 45 0 -
Peak Hour Factor Heavy Vehicles, % Mvmt Flow Major/Minor Conflicting Flow All Stage 1 Stage 2 84 Critical Hdwy 6. Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Stage 1 Mov Cap-2 Maneuver Stage 1 Stage 1 68	35 0 49 24 75 49 .4 .4	85 0 15 376 - - 6.2	94 0 10 Major1 397 - 4.1	94 1 828 0 - -	91 2 351 Major2 - - -	91 0 45 0 - -
Major/Minor Minor Conflicting Flow All 122 Stage 1 37 Stage 2 84 Critical Hdwy 6. Critical Hdwy Stg 1 5. Critical Hdwy Stg 2 5. Follow-up Hdwy 3. Pot Cap-1 Maneuver Stage 1 69 Stage 2 42 Platoon blocked, % Mov Cap-1 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68	0 49 r2 24 75 49 .4	0 15 376 - - 6.2 -	0 10 Major1 397 - - 4.1	1 828 0 - -	2 351 Major2 - - - -	0 45 0 - -
Mymt Flow Major/Minor Conflicting Flow All Stage 1 Stage 2 84 Critical Hdwy 6. Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy 3. Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver 19 Stage 1 68	r224	15 376 - - 6.2 -	10 Major1 397 - - 4.1	828 0 - -	351 Major2 - - -	0 -
Major/Minor Minor Conflicting Flow All 122 Stage 1 37 Stage 2 84 Critical Hdwy 6. Critical Hdwy Stg 1 5. Critical Hdwy Stg 2 5. Follow-up Hdwy 3. Pot Cap-1 Maneuver Stage 1 69 Stage 2 42 Platoon blocked, % Mov Cap-1 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68	r2 24 75 49 .4 .4	376 - - 6.2 -	Major1 397 - - 4.1	0 - -	Major2 - - - -	0 -
Conflicting Flow All 122 Stage 1 37 Stage 2 84 Critical Hdwy 6. Critical Hdwy Stg 1 5. Critical Hdwy Stg 2 5. Follow-up Hdwy 3. Pot Cap-1 Maneuver 20 Stage 1 69 Stage 2 42 Platoon blocked, % Mov Cap-1 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68	24 75 19 .4 .4	376 - - 6.2 -	397 - - 4.1 -	0 - - -	- - - -	- - -
Conflicting Flow All 122 Stage 1 37 Stage 2 84 Critical Hdwy 6. Critical Hdwy Stg 1 5. Critical Hdwy Stg 2 5. Follow-up Hdwy 3. Pot Cap-1 Maneuver 20 Stage 1 69 Stage 2 42 Platoon blocked, % Mov Cap-1 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68	24 75 19 .4 .4	376 - - 6.2 -	397 - - 4.1 -	0 - - -	- - - -	- - -
Conflicting Flow All 122 Stage 1 37 Stage 2 84 Critical Hdwy 6. Critical Hdwy Stg 1 5. Critical Hdwy Stg 2 5. Follow-up Hdwy 3. Pot Cap-1 Maneuver 20 Stage 1 69 Stage 2 42 Platoon blocked, % Mov Cap-1 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68	24 75 19 .4 .4	376 - - 6.2 -	397 - - 4.1	0 - - -	- - - -	- - -
Stage 1 37 Stage 2 84 Critical Hdwy 6. Critical Hdwy Stg 1 5. Critical Hdwy Stg 2 5. Follow-up Hdwy 3. Pot Cap-1 Maneuver 20 Stage 1 69 Stage 2 42 Platoon blocked, % Mov Cap-1 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68	75 19 .4 .4 .4	- 6.2 -	- - 4.1 -	- - -	- - -	- - -
Stage 2 84 Critical Hdwy 6. Critical Hdwy Stg 1 5. Critical Hdwy Stg 2 5. Follow-up Hdwy 3. Pot Cap-1 Maneuver 20 Stage 1 69 Stage 2 42 Platoon blocked, % Mov Cap-1 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68	19 .4 .4 .4	- 6.2 - -	4.1 -	-	- - -	-
Critical Hdwy 6. Critical Hdwy Stg 1 5. Critical Hdwy Stg 2 5. Follow-up Hdwy 3. Pot Cap-1 Maneuver 20 Stage 1 69 Stage 2 42 Platoon blocked, % Mov Cap-1 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68	.4 .4 .4	6.2 -	4.1 -	-	- -	-
Critical Hdwy Stg 1 5. Critical Hdwy Stg 2 5. Follow-up Hdwy 3. Pot Cap-1 Maneuver 20 Stage 1 69 Stage 2 42 Platoon blocked, % Mov Cap-1 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68	.4 .4	-	-	-	-	-
Critical Hdwy Stg 2 5. Follow-up Hdwy 3. Pot Cap-1 Maneuver 20 Stage 1 69 Stage 2 42 Platoon blocked, % Mov Cap-1 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68	.4	-		-		-
Follow-up Hdwy 3. Pot Cap-1 Maneuver 20 Stage 1 69 Stage 2 42 Platoon blocked, % Mov Cap-1 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68			-	-	_	
Pot Cap-1 Maneuver 20 Stage 1 69 Stage 2 42 Platoon blocked, % Mov Cap-1 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68	5					-
Stage 1 69 Stage 2 42 Platoon blocked, % Mov Cap-1 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68		3.3	2.2	-	-	-
Stage 2 42 Platoon blocked, % Mov Cap-1 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68		675	1173	-	-	-
Platoon blocked, % Mov Cap-1 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68		-	-	-	-	-
Mov Cap-1 Maneuver 19 Mov Cap-2 Maneuver 19 Stage 1 68	23	-	-	-	-	-
Mov Cap-2 Maneuver 19 Stage 1 68				-	-	-
Stage 1 68	96	674	1172	-	-	-
•	96	-	-	-	-	-
Stage 2 42	37	-	-	-	-	-
J	23	-	-	-	_	-
	-		ND		0.5	
	В		NB		SB	
HCM Control Delay, s/v 65.	.3		0.1		0	
HCM LOS	F					
Minor Lane/Major Mvmt		NBL	NPT	EBLn1	SBT	SBR
Capacity (veh/h)		1172	-		-	-
HCM Lane V/C Ratio		0.008		0.784	-	-
HCM Control Delay (s/veh)	U.	8.1	0		-	-
HCM Lane LOS	U.	Α	Α	F	-	-
HCM 95th %tile Q (veh)	Ü.	0	-	5.5	-	-

Intersection						
Int Delay, s/veh	0.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	₩.	LDIX	NDL	4	<u>361</u>	ODIX
Traffic Vol, veh/h	T	11	26	797	351	2
	2	11	26		351	2
Future Vol, veh/h	3			797		
Conflicting Peds, #/hr	-	3	3	0	0	3
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	-	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	65	65	90	90	90	90
Heavy Vehicles, %	0	0	0	1	2	0
Mvmt Flow	3	17	29	886	390	2
Major/Minor	Minor2	N	Major1	N	/lajor2	
Conflicting Flow All	1341	397	395	0	-	0
Stage 1	394	-	-	-	-	-
Stage 2	947	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	_	_	_
Critical Hdwy Stg 1	5.4	-	-	_	_	_
Critical Hdwy Stg 2	5.4	_	_	_	_	_
Follow-up Hdwy	3.5	3.3	2.2	_	<u>-</u>	_
	170	657	1175	-		
Pot Cap-1 Maneuver			11/5	-	-	-
Stage 1	686	-	-	-	-	-
Stage 2	380	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	161	654	1172	-	-	-
Mov Cap-2 Maneuver	161	-	-	-	-	-
Stage 1	651	-	-	-	-	-
Stage 2	379	-	-	-	_	-
g 	J. J					
Approach	EB		NB		SB	
HCM Control Delay, s/	v 13.5		0.3		0	
HCM LOS	В					
Minor Lane/Major Mvn	nt	NBL	NRT	EBLn1	SBT	SBR
	IL					אמט
Capacity (veh/h)		1172	-		-	
HCM Lane V/C Ratio		0.025		0.045	-	-
HCM Control Delay (s/	veh)	8.1	0	13.5	-	-
HCM Lane LOS		Α	Α	В	-	-
HCM 95th %tile Q (vel	1)	0.1	-	0.1	-	-

Intersection Delay, s/veh 8.1 Intersection LOS A	Intersection			
	Intersection Delay, s/veh	8.1		
***************************************	Intersection LOS	Α		

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	12	5	10	18	3	4	118	15	1	49	4
Future Vol, veh/h	1	12	5	10	18	3	4	118	15	1	49	4
Peak Hour Factor	0.45	0.45	0.45	0.43	0.43	0.43	0.71	0.71	0.71	0.84	0.84	0.84
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	2	27	11	23	42	7	6	166	21	1	58	5
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay, s/veh	7.6			8			8.3			7.7		
HCM LOS	Α			Α			Α			Α		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	3%	6%	32%	2%	
Vol Thru, %	86%	67%	58%	91%	
Vol Right, %	11%	28%	10%	7%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	137	18	31	54	
LT Vol	4	1	10	1	
Through Vol	118	12	18	49	
RT Vol	15	5	3	4	
Lane Flow Rate	193	40	72	64	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.219	0.049	0.09	0.077	
Departure Headway (Hd)	4.084	4.394	4.517	4.317	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Сар	864	819	797	833	
Service Time	2.178	2.4	2.523	2.325	
HCM Lane V/C Ratio	0.223	0.049	0.09	0.077	
HCM Control Delay, s/veh	8.3	7.6	8	7.7	
HCM Lane LOS	Α	Α	Α	Α	
HCM 95th-tile Q	0.8	0.2	0.3	0.2	

Intersection						
Int Delay, s/veh	0.9					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	À	4	40	4	-	^
Traffic Vol, veh/h	8	4	10	139	54	8
Future Vol, veh/h	8	4	10	139	54	8
Conflicting Peds, #/hr	0	0	0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	75	75	96	96	86	86
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	11	5	10	145	63	9
N.A. ' (N.A' N.	4: 0				4 : 0	
	linor2		Major1		/lajor2	_
Conflicting Flow All	233	68	72	0	-	0
Stage 1	68	-	-	-	-	-
Stage 2	165	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	760	1001	1541	-	-	-
Stage 1	960	-	_	-	_	-
Stage 2	869	_	-	-	_	_
Platoon blocked, %				_	_	_
Mov Cap-1 Maneuver	755	1001	1541	_	_	_
Mov Cap-1 Maneuver	755	-	1071	_	_	
Stage 1	953	-	-	_	-	_
	869	-	-	_	-	•
Stage 2	009	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s/v	9.5		0.5		0	
HCM LOS	A		0.0			
110111 200						
Minor Lane/Major Mvmt		NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1541	-	822	-	-
HCM Lane V/C Ratio		0.007	-	0.019	-	-
HCM Control Delay (s/v	eh)	7.4	0	9.5	-	-
HCM Lane LOS	•	Α	Α	Α	-	-
HCM 95th %tile Q (veh)		0	-	0.1	-	-
(VOII)				• • •		

Intersection						
Int Delay, s/veh	5.5					
Movement I	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽			स	¥	
Traffic Vol, veh/h	3	0	11	1	1	5
Future Vol, veh/h	3	0	11	1	1	5
Conflicting Peds, #/hr	0	0	0	0	0	0
•	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	_	None	-	None
Storage Length	_	-	_	-	0	-
Veh in Median Storage, #	<i>‡</i> 0	_	_	0	0	_
Grade, %	0	_	_	0	0	_
Peak Hour Factor	38	38	60	60	75	75
Heavy Vehicles, %	0	0	0	0	0	0
Mymt Flow	8	0	18	2	1	7
IVIVIIIL I IOW	U	U	10		1	1
Major/Minor Ma	ajor1	N	//ajor2	N	Minor1	
Conflicting Flow All	0	0	8	0	46	8
Stage 1	-	-	-	-	8	-
Stage 2	-	-	-	-	38	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	_	_	2.2	_	3.5	3.3
Pot Cap-1 Maneuver	_	-	1625	-	969	1080
Stage 1	_	_	_	_	1020	_
Stage 2	-	-	-	-	990	-
Platoon blocked, %	_	<u>-</u>		_		
Mov Cap-1 Maneuver	_	_	1625	_	958	1080
Mov Cap-2 Maneuver	_	_	-	_	958	-
Stage 1	_	_	_	_	1020	_
Stage 2	_	_	_	_	979	_
Stage 2	_		-	_	313	_
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		6.6		8.4	
HCM LOS					Α	
		IDI 4	EDT	ED5	14/51	MET
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		1058	-	-		-
HCM Lane V/C Ratio		0.008	-		0.011	-
HCM Control Delay (s/vel	h)	8.4	-	-	7.2	0
HCM Lane LOS HCM 95th %tile Q (veh)		A 0	-	-	A 0	Α

Intersection						
Int Delay, s/veh	1.2					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥	_	^	4	}	400
Traffic Vol, veh/h	41	5	0	200	912	130
Future Vol, veh/h	41	5	0	200	912	130
Conflicting Peds, #/hr	4	4	4	_ 0	_ 0	_ 4
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	93	93	82	82	85	85
Heavy Vehicles, %	0	0	0	5	1	0
Mvmt Flow	44	5	0	244	1073	153
Major/Minor N	Minor2	N	Major1	ı	/lajor2	
Conflicting Flow All	1402	1158	1230	0	- najoiz	0
Stage 1	1154	-	1230	-	_	-
Stage 2	248	_		_	_	_
	6.4	6.2	4.1			_
Critical Hdwy	5.4		4.1	-	-	-
Critical Hdwy Stg 1		-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	156	241	574	-	-	-
Stage 1	303	-	-	-	-	-
Stage 2	798	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	154	239	571	-	-	-
Mov Cap-2 Maneuver	154	-	-	-	-	-
Stage 1	301	-	-	-	-	-
Stage 2	794	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s/\			0		0	
HCM LOS	7 37.3 E		U		U	
I IOIVI LOS						
Minor Lane/Major Mvm	t	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		571	-	160	-	-
HCM Lane V/C Ratio		-	-	0.309	-	-
HCM Control Delay (s/\	/eh)	0	-		-	-
HCM Lane LOS	,	Α	-	Е	-	-
HCM 95th %tile Q (veh)	0	-	1.2	-	-
	,					

Intersection						
Int Delay, s/veh	1					
	EDI	EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y		4.0	4	4	
Traffic Vol, veh/h	0	28	12	223	905	2
Future Vol, veh/h	0	28	12	223	905	2
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	_	-	0	0	-
Peak Hour Factor	50	50	81	81	80	80
Heavy Vehicles, %	0	8	0	4	2	0
Mvmt Flow	0	56	15	275	1131	3
Major/Minor	Minor2		laior1		Major2	
			Major1			
Conflicting Flow All	1438	1133	1134	0	-	0
Stage 1	1133	-	-	-	-	-
Stage 2	305	-	-	-	-	-
Critical Hdwy	6.4	6.28	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy		3.372	2.2	-	-	-
Pot Cap-1 Maneuver	148	240	623	-	-	-
Stage 1	310	-	-	-	-	-
Stage 2	752	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	144	240	623	-	-	-
Mov Cap-2 Maneuver	144	-	-	-	-	-
Stage 1	301	-	-	-	-	-
Stage 2	752	-	-	-	-	-
Annragah	ED		ND		CD	
Approach	EB		NB		SB	
HCM Control Delay, s/			0.6		0	
HCM LOS	С					
Minor Lane/Major Mvm	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		623	-		-	-
HCM Lane V/C Ratio		0.024		0.233	_	_
HCM Control Delay (s/	(veh)	10.9	0		-	_
HCM Lane LOS	vonj	10.9 B	A	24.3 C	_	_
HCM 95th %tile Q (veh	۱)	0.1	-	0.9	-	_
HOW JOHN JOHNE Q (VEI	'/	0.1		0.9	_	_

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	3	10	8	18	1	2	3	72	12	3	134	2
Future Vol, veh/h	3	10	8	18	1	2	3	72	12	3	134	2
Peak Hour Factor	0.59	0.59	0.59	0.39	0.39	0.39	0.90	0.90	0.90	0.74	0.74	0.74
Heavy Vehicles, %	33	0	0	12	0	0	33	0	18	0	0	0
Mvmt Flow	5	17	14	46	3	5	3	80	13	4	181	3
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay, s/veh	8.3			8.3			8.5			8.5		
HCM LOS	Α			Α			Α			Α		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	3%	14%	86%	2%	
Vol Thru, %	83%	48%	5%	96%	
Vol Right, %	14%	38%	10%	1%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	87	21	21	139	
LT Vol	3	3	18	3	
Through Vol	72	10	1	134	
RT Vol	12	8	2	2	
Lane Flow Rate	97	36	54	188	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.129	0.049	0.074	0.221	
Departure Headway (Hd)	4.807	4.985	4.917	4.236	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Сар	748	720	730	851	
Service Time	2.822	3.005	2.936	2.249	
HCM Lane V/C Ratio	0.13	0.05	0.074	0.221	
HCM Control Delay, s/veh	8.5	8.3	8.3	8.5	
HCM Lane LOS	Α	Α	Α	Α	
HCM 95th-tile Q	0.4	0.2	0.2	0.8	

Intersection						
Int Delay, s/veh	1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥	ופם	TIDE	4	<u>₩</u>	אופט
Traffic Vol, veh/h	14	10	2	70	156	8
Future Vol, veh/h	14	10	2	70	156	8
· · · · · · · · · · · · · · · · · · ·	0	0	0	0		0
Conflicting Peds, #/hr					0	
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	79	79	86	86	82	82
Heavy Vehicles, %	15	0	0	2	1	0
Mvmt Flow	18	13	2	81	190	10
	Minor2		Major1	N	/lajor2	
Conflicting Flow All	280	195	200	0	-	0
Stage 1	195	-	-	-	-	-
Stage 2	85	-	-	-	-	-
Critical Hdwy	6.55	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.55	_	_	_	-	_
Critical Hdwy Stg 2	5.55	_	_	_	_	_
Follow-up Hdwy	3.635	3.3	2.2	_	_	_
Pot Cap-1 Maneuver	683	851	1384	-	_	_
•	808	-	1304	_		
Stage 1			-	-	-	-
Stage 2	907	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	682	851	1384	-	-	-
Mov Cap-2 Maneuver	682	_		-	-	-
Stage 1	806	-	-	-	-	-
Stage 2	907	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s/	v 10		0.2		0	
HCM LOS	В					
Minor Long/Major M.	.	NDI	NDT	EDI 51	CDT	CDD
Minor Lane/Major Mvn	IL	NBL		EBLn1	SBT	SBR
Capacity (veh/h)		1384	-		-	-
HCM Lane V/C Ratio		0.002		0.041	-	-
HCM Control Delay (sa	veh)	7.6	0	10	-	-
HCM Lane LOS		Α	Α	В	-	-
HCM 95th %tile Q (vel	1)	0	-	0.1	-	-
	•					

Intersection						
Int Delay, s/veh	7					
	EBT	EBR	WBL	WBT	NBL	NBR
		EDK	VVDL		INDL	INDK
Lane Configurations	1	۸	0	र्च		24
Traffic Vol, veh/h	2	0	9	2	0	24
Future Vol, veh/h	2	0	9	2	0	24
Conflicting Peds, #/hr	0	0	0	_ 0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	50	50	50	50	69	69
Heavy Vehicles, %	0	0	0	0	0	9
Mvmt Flow	4	0	18	4	0	35
Major/Minor Ma	ajor1	N	//ajor2	N	Minor1	
Conflicting Flow All	0	0	4	0	44	4
Stage 1	-	U	-	-	4	-
Stage 2	_	_	_	_	40	-
	-	-	4.1		6.4	6.29
Critical House Star 1	-	-			5.4	0.29
Critical Holy Stg 1	-	-	-	-		
Critical Hdwy Stg 2	-	-	-	-	5.4	- 204
Follow-up Hdwy	-	-	2.2	-	3.5	3.381
Pot Cap-1 Maneuver	-	-	1631	-	972	1059
Stage 1	-	-	-	-	1024	-
Stage 2	-	-	-	-	988	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1631	-	961	1059
Mov Cap-2 Maneuver	-	-	-	-	961	-
Stage 1	-	-	-	-	1024	-
Stage 2	-	-	-	-	977	-
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		5.9		8.5	
HCM LOS	U		J.3		0.5 A	
I IOIVI LOG					А	
Minor Lane/Major Mvmt	١	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		1059	-	-	1631	-
HCM Lane V/C Ratio		0.033	-	-	0.011	-
HCM Control Delay (s/ve	h)	8.5	-	-	7.2	0
HCM Lane LOS		Α	-	-	Α	Α
HCM 95th %tile Q (veh)		0.1	-	-	0	-
HOW JOHN JOHN (VEII)		0.1		_	U	_

section telay, s/veh 17.9 tement EBL EBR NBL NBT SBT SBR c Configurations ic Vol, veh/h 141 14 10 877 369 48 re Vol, veh/h 141 14 10 877 369 48 flicting Peds, #hr 1 1 1 0 0 0 1 Control Stop Stop Free Free Free Free Free Pree Pree Pree
Configurations Fic Vol, veh/h 141 14 10 877 369 48 Fre Vol, veh/h 141 14 10 877 369 48 Fre Vol, veh/h 141 14 10 877 369 48 Fre Vol, veh/h 141 14 10 0 0 1 Control Stop Stop Free Free Free Free Channelized - None age Length 0
Configurations Fic Vol, veh/h 141 14 10 877 369 48 Fre Vol, veh/h 141 14 10 877 369 48 Fre Vol, veh/h 141 14 10 877 369 48 Fre Vol, veh/h 141 14 10 0 0 1 Control Stop Stop Free Free Free Free Channelized - None age Length 0
fic Vol, veh/h 141 14 10 877 369 48 re Vol, veh/h 141 14 10 877 369 48 liicting Peds, #/hr 1 1 1 1 0 0 1 Control Stop Stop Free Free Free Free Channelized - None - None - None age Length 0
re Vol, veh/h 141 14 10 877 369 48 flicting Peds, #/hr 1 1 1 1 0 0 1 Control Stop Stop Free Free Free Free Channelized - None - None - None age Length 0
Find the property of the prope
Control Stop Channelized Stop Channelized Free None
Channelized - None - None - None age Length 0 - 0 - 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
age Length 0
in Median Storage, # 0
te, % 0 0 0 0 - c Hour Factor 85 85 94 94 91 91 vy Vehicles, % 0 0 0 1 2 0 out Flow 166 16 11 933 405 53 out Flow 166 16 11 933 405 53 out Flow 189 434 459 0 - 0 Stage 1 433
Hour Factor
ry Vehicles, % 0 0 0 1 2 0 Int Flow 166 16 11 933 405 53 Interpretation of the state of the st
nt Flow 166 16 11 933 405 53 Interpretation Minor Minor Major Major
Minor Minor Major Major Major Major Major Major Major Major Minor Minor Minor Major Major Minor Minor Major
Stage 1
Stage 1
Stage 1 433 -
Stage 2 956
cal Hdwy Stg 1 5.4
cal Hdwy Stg 1 5.4
cal Hdwy Stg 2 5.4
Stage 1 658
Cap-1 Maneuver ~ 159 626 1113 - - - Stage 1 658 - - - - Stage 2 376 - - - - Stage 3 - - - - - Cap-1 Maneuver ~ 155 625 1112 - - - Cap-2 Maneuver ~ 155 - - - - - Stage 1 644 - - - - -
Stage 1 658 -
Stage 2 376 - - - - con blocked, % - - - - Cap-1 Maneuver ~ 155 625 1112 - - - Cap-2 Maneuver ~ 155 - - - - Stage 1 644 - - - -
on blocked, %
Cap-1 Maneuver ~ 155 625 1112
Cap-2 Maneuver ~ 155 Stage 1 644
Stage 1 644
· · · · · · · · · · · · · · · · · · ·
Stage 2 376
roach EB NB SB
1 Control Delay, s/v154.7 0.1 0
1LOS F
or Lane/Major Mvmt NBL NBT EBLn1 SBT SBR
acity (veh/h) 1112 - 166
1 Lane V/C Ratio 0.01 - 1.099
1 Control Delay (s/veh) 8.3 0 154.7
1 Lane LOS A A F
1 95th %tile Q (veh) 0 - 9.4
es
olume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

Intersection						
Int Delay, s/veh	0.4					
•		===			057	055
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	À			4	\$	
Traffic Vol, veh/h	2	12	28	897	403	2
Future Vol, veh/h	2	12	28	897	403	2
Conflicting Peds, #/hr	3	3	3	0	0	3
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	65	65	90	90	90	90
Heavy Vehicles, %	0	0	0	1	2	0
Mvmt Flow	3	18	31	997	448	2
		-				
	Minor2		//ajor1		//ajor2	
Conflicting Flow All	1514	455	453	0	-	0
Stage 1	452	-	-	-	-	-
Stage 2	1062	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	133	609	1118	-	-	-
Stage 1	645	-	_	-	_	-
Stage 2	335	-	_	-	-	-
Platoon blocked, %				-	_	-
Mov Cap-1 Maneuver	124	606	1115	_	-	_
Mov Cap-1 Maneuver	124	-		_	<u>-</u>	_
Stage 1	604	_				_
Stage 2	334	_	_	_	_	_
Staye 2	334	<u>-</u>	-	<u>-</u>	<u>-</u>	-
Approach	EB		NB		SB	
HCM Control Delay, s/	v 14.8		0.3		0	
HCM LOS	В					
NA: 1 /NA		NDI	Not	EDL 4	ODT	000
NUMBER LONG/MOIOR MAN	nr i	NBL	NRT	EBLn1	SBT	SBR
Minor Lane/Major Mvm						
Capacity (veh/h)		1115	-		-	-
Capacity (veh/h) HCM Lane V/C Ratio		1115 0.028	-	0.055	-	-
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s/		1115 0.028 8.3	- 0	0.055 14.8		
Capacity (veh/h) HCM Lane V/C Ratio	/veh)	1115 0.028	-	0.055	-	-

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	13	5	11	19	3	4	132	16	1	57	4
Future Vol, veh/h	1	13	5	11	19	3	4	132	16	1	57	4
Peak Hour Factor	0.45	0.45	0.45	0.43	0.43	0.43	0.71	0.71	0.71	0.84	0.84	0.84
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	2	29	11	26	44	7	6	186	23	1	68	5
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay, s/veh	7.8			8.1			8.6			7.8		
HCM LOS	Α			Α			Α			Α		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	3%	5%	33%	2%	
Vol Thru, %	87%	68%	58%	92%	
Vol Right, %	11%	26%	9%	6%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	152	19	33	62	
LT Vol	4	1	11	1	
Through Vol	132	13	19	57	
RT Vol	16	5	3	4	
Lane Flow Rate	214	42	77	74	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.251	0.053	0.098	0.09	
Departure Headway (Hd)	4.216	4.48	4.597	4.367	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	856	800	780	822	
Service Time	2.216	2.503	2.618	2.383	
HCM Lane V/C Ratio	0.25	0.053	0.099	0.09	
HCM Control Delay, s/veh	8.6	7.8	8.1	7.8	
HCM Lane LOS	Α	Α	Α	Α	
HCM 95th-tile Q	1	0.2	0.3	0.3	

Intersection						
Int Delay, s/veh	0.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	₩.	LDI	NDL	ND1		אמט
Traffic Vol, veh/h	""	4	11	€ 154	1 → 62	9
					62	
Future Vol, veh/h	9	0	11	154	0	9
Conflicting Peds, #/hr			0 Eroo	0 Eroo		
Sign Control RT Channelized	Stop	Stop	Free	Free	Free -	Free
	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	- 75	-	0	0	-
Peak Hour Factor	75	75	96	96	86	86
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	12	5	11	160	72	10
Major/Minor N	1inor2	N	/lajor1	N	Major2	
Conflicting Flow All	259	77	82	0	-	0
Stage 1	77	-	-	-	_	-
Stage 2	182	_	_		_	_
Critical Hdwy	6.4	6.2	4.1	<u>-</u>	-	-
Critical Hdwy Stg 1	5.4	0.2	4.1	-	_	_
	5.4	-	-	-	-	-
Critical Hdwy Stg 2			2.2	-	=	_
Follow-up Hdwy	3.5	3.3		-	-	-
Pot Cap-1 Maneuver	734	990	1528	-	-	-
Stage 1	951	-	-	-	-	-
Stage 2	854	-	-	-	-	-
Platoon blocked, %		000	4=00	-	-	-
Mov Cap-1 Maneuver	728	990	1528	-	-	-
Mov Cap-2 Maneuver	728	-	-	-	-	-
Stage 1	943	-	-	-	-	-
Stage 2	854	-	-	-	-	-
Approach	EB		NB		SB	
			0.5		0	
HCM Control Delay, s/v			0.5		U	
HCM LOS	Α					
Minor Lane/Major Mvmt		NBL	NBT E	EBLn1	SBT	SBR
Capacity (veh/h)		1528	_		_	_
HCM Lane V/C Ratio		0.007		0.022	_	_
HCM Control Delay (s/v	eh)	7.4	0	9.6	-	_
HCM Lane LOS	J/	A	A	A	_	_
HCM 95th %tile Q (veh)		0	-	0.1	_	_
				0.1		

Intersection						
Int Delay, s/veh	5.7					
		EDD	WDI	WDT	NDI	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	- ♣	^	40	નુ	Y	-
Traffic Vol, veh/h	3	0	12	1	1	5
Future Vol, veh/h	3	0	12	1	1	5
Conflicting Peds, #/hr	0	0	0	0	0	0
3	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	ŧ 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	38	38	60	60	75	75
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	8	0	20	2	1	7
	ajor1		//ajor2		/linor1	
Conflicting Flow All	0	0	8	0	50	8
Stage 1	-	-	-	-	8	-
Stage 2	-	-	-	-	42	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	_	-	1625	-	964	1080
Stage 1	-	_	_	_	1020	_
Stage 2	_	_	_	_	986	_
Platoon blocked, %	_	_		_	000	
Mov Cap-1 Maneuver	_	_	1625	_	952	1080
Mov Cap-2 Maneuver		-		-	952	1000
	-	-	-		1020	
Stage 1	-	-	-	-		-
Stage 2	-	-	-	-	974	-
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		6.7		8.4	
HCM LOS	•		V.,		A	
HOW LOO					Λ.	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		1056	-	-	1625	-
HCM Lane V/C Ratio		800.0	-	-	0.012	-
HCM Control Delay (s/ve	h)	8.4	-	-	7.2	0
HCM Lane LOS		Α	-	-	Α	Α
HCM 95th %tile Q (veh)		0	-	-	0	-

HCM 6th TWSC DCL Vanasse & Associates

Intersection						
Int Delay, s/veh	1.2					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	\Y		^	4	∱	120
Traffic Vol, veh/h	42	5	0	200	912	130
Future Vol, veh/h	42	5	0	200	912	130
Conflicting Peds, #/hr	4	4	4	_ 0	_ 0	_ 4
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storag		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	93	93	82	82	85	85
Heavy Vehicles, %	0	0	0	5	1	0
Mvmt Flow	45	5	0	244	1073	153
Major/Minor	Minor2	N	Major1	N	/lajor2	
			Major1			
Conflicting Flow All	1402		1230	0	-	0
Stage 1	1154	-	-	-	-	-
Stage 2	248	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	156	241	574	-	-	-
Stage 1	303	-	-	-	-	-
Stage 2	798	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	154	239	571	-	-	-
Mov Cap-2 Maneuver	154	-	-	-	-	-
Stage 1	301	-	-	-	-	-
Stage 2	794	-	-	-	-	-
						
					-	
Approach	EB		NB		SB	
HCM Control Delay, s			0		0	
HCM LOS	Е					
Minor Lane/Major Mvr	nt	NBL	NRT	EBLn1	SBT	SBR
	п					אומט
Capacity (veh/h)		571	-		-	-
HCM Control Doloy (/, , o lo \	-		0.316	-	-
HCM Control Delay (s	/ven)	0	-		-	-
HCM Lane LOS	1.\	A	-	E	-	-
HCM 95th %tile Q (ve	n)	0	-	1.3	-	-

HCM 6th TWSC DCL Vanasse & Associates

Intersection						
Int Delay, s/veh	1.2					
	EDI	EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			4	\$	
Traffic Vol, veh/h	0	32	13	223	905	2
Future Vol, veh/h	0	32	13	223	905	2
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	_	-	0	0	_
Peak Hour Factor	50	50	81	81	80	80
Heavy Vehicles, %	0	8	0	4	2	0
Mvmt Flow	0	64	16	275	1131	3
IVIVIII I IOVV	U	04	10	210	1101	3
Major/Minor	Minor2	N	//ajor1	N	/lajor2	
Conflicting Flow All	1440	1133	1134	0	-	0
Stage 1	1133	-	-	-	-	_
Stage 2	307	_	_	_	_	_
Critical Hdwy	6.4	6.28	4.1	_	_	_
Critical Hdwy Stg 1	5.4	0.20	7.1		_	
	5.4		-	-		-
Critical Hdwy Stg 2		-	-	-	-	-
Follow-up Hdwy	3.5	3.372	2.2	-	-	-
Pot Cap-1 Maneuver	148	240	623	-	-	-
Stage 1	310	-	-	-	-	-
Stage 2	751	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	144	240	623	-	-	-
Mov Cap-2 Maneuver	144	_	-	-	-	_
Stage 1	301	-	_	_	_	_
Stage 2	751	<u>-</u>	_	_	_	_
Olage 2	701					_
Approach	EB		NB		SB	
HCM Control Delay, s/	v 25.4		0.6		0	
HCM LOS	D					
				-DI 1	05-	055
Minor Lane/Major Mvm	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		623	-		-	-
HCM Lane V/C Ratio		0.026	-	0.267	-	-
HCM Control Delay (s/	veh)	10.9	0	25.4	-	-
HCM Lane LOS		В	Α	D	-	-
HCM 95th %tile Q (veh	1)	0.1	_	1	_	-
	7	J.,				

Intersection Delay, s/veh 8.5 Intersection LOS A	Intersection			
Intersection LOS A	Intersection Delay, s/veh	8.5		
	Intersection LOS	Α		

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	4	11	9	18	2	2	3	72	15	3	134	2
Future Vol, veh/h	4	11	9	18	2	2	3	72	15	3	134	2
Peak Hour Factor	0.59	0.59	0.59	0.39	0.39	0.39	0.90	0.90	0.90	0.74	0.74	0.74
Heavy Vehicles, %	33	0	0	12	0	0	33	0	18	0	0	0
Mvmt Flow	7	19	15	46	5	5	3	80	17	4	181	3
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay, s/veh	8.3			8.4			8.6			8.5		
HCM LOS	Α			Α			Α			Α		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	3%	17%	82%	2%	
Vol Thru, %	80%	46%	9%	96%	
Vol Right, %	17%	38%	9%	1%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	90	24	22	139	
LT Vol	3	4	18	3	
Through Vol	72	11	2	134	
RT Vol	15	9	2	2	
Lane Flow Rate	100	41	56	188	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.134	0.057	0.077	0.222	
Departure Headway (Hd)	4.81	5.005	4.928	4.26	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	747	717	728	845	
Service Time	2.828	3.027	2.95	2.275	
HCM Lane V/C Ratio	0.134	0.057	0.077	0.222	
HCM Control Delay, s/veh	8.6	8.3	8.4	8.5	
HCM Lane LOS	Α	Α	Α	Α	
HCM 95th-tile Q	0.5	0.2	0.2	8.0	

Intersection						
Int Delay, s/veh	1.2					
•		===			057	055
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			4	₽	
Traffic Vol, veh/h	17	11	3	70	157	8
Future Vol, veh/h	17	11	3	70	157	8
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	79	79	86	86	82	82
Heavy Vehicles, %	15	0	0	2	1	0
Mvmt Flow	22	14	3	81	191	10
Major/Minor	Minor2	ı	/lajor1	N	/lajor2	
Conflicting Flow All	283	196	201	0	-	0
Stage 1	196	-	-	-	-	-
Stage 2	87	-	-	-	-	-
Critical Hdwy	6.55	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.55	-	-	-	-	-
Critical Hdwy Stg 2	5.55	-	-	-	-	-
Follow-up Hdwy	3.635	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	680	850	1383	-	-	-
Stage 1	807	-	-	-	-	-
Stage 2	905	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	679	850	1383	-	-	
Mov Cap-2 Maneuver	679	-	-	-	-	-
Stage 1	805	-	-	-	-	-
Stage 2	905	-	-	-	-	-
J -						
Approach	EB		NB		SB	
HCM Control Delay, s/			0.3		0	
HCM LOS	V 10.1		0.3		U	
I IOIVI LOS	В					
Minor Lane/Major Mvm	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1383	-	737	-	-
HCM Lane V/C Ratio		0.003	-	0.048	-	-
HCM Control Delay (s/	veh)	7.6	0	10.1	-	-
HCM Lane LOS		A	A	В	_	-
HCM 95th %tile Q (veh	1)	0	-	0.2	-	-
	1					

Intersection						
Int Delay, s/veh	7					
		- CDD	WDI	MOT	NDI	NDD
	BT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	Þ			र्न	Ą	
Traffic Vol, veh/h	2	0	9	2	0	24
Future Vol, veh/h	2	0	9	2	0	24
Conflicting Peds, #/hr	0	0	0	0	0	0
0	ree	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	_
Peak Hour Factor	50	50	50	50	69	69
Heavy Vehicles, %	0	0	0	0	0	9
Mvmt Flow	4	0	18	4	0	35
INIVITIC I IOW	Т	-	10	7	U	00
Major/Minor Maj	or1	N	//ajor2	N	Minor1	
Conflicting Flow All	0	0	4	0	44	4
Stage 1	-	-	-	-	4	-
Stage 2	-	_	_	_	40	_
Critical Hdwy	_	_	4.1	_	6.4	6.29
Critical Hdwy Stg 1	_	_	-	_	5.4	-
Critical Hdwy Stg 2	_	_	_	_	5.4	_
	_	_	2.2	-		3.381
Follow-up Hdwy	_					
Pot Cap-1 Maneuver	-	-	1631	-	972	1059
Stage 1	-	-	-	-	1024	-
Stage 2	-	-	-	-	988	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1631	-	961	1059
Mov Cap-2 Maneuver	-	-	-	-	961	-
Stage 1	-	-	-	-	1024	-
Stage 2	-	-	-	_	977	-
g- -						
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		5.9		8.5	
HCM LOS					Α	
Minor Long/Maior Mysset		JDI 1	CDT	EDD	WDI	WDT
Minor Lane/Major Mvmt	ľ	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		1059	-		1631	-
HCM Lane V/C Ratio		0.033	-	-	0.011	-
HCM Control Delay (s/veh	1)	8.5	-	-	7.2	0
HCM Lane LOS		Α	-	-	Α	Α
HCM 95th %tile Q (veh)		0.1	-	-	0	-
,						

HCM 6th TWSC DCL Vanasse & Associates

Intersection						
Int Delay, s/veh	1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
		EDK	VVDL			NDK
Lane Configurations	}	0	4	र्स्	¥	2
Traffic Vol, veh/h	21	0	1	6	0	3
Future Vol, veh/h	21	0	1	6	0	3
Conflicting Peds, #/hr	0	_ 0	_ 0	_ 0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	ŧ 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	23	0	1	7	0	3
NA - ' /NA' NA -	4		4 - ' - 0		M'	
	ajor1		Major2		Minor1	
Conflicting Flow All	0	0	23	0	32	23
Stage 1	-	-	-	-	23	-
Stage 2	-	-	-	-	9	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1592	-	982	1054
Stage 1	-	-	-	-	1000	-
Stage 2	-	-	_	-	1014	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	_	_	1592	-	981	1054
Mov Cap-2 Maneuver	_	_	-	_	981	-
Stage 1	_	_	_	_	1000	_
Stage 2			_		1013	_
Olago Z	_			_	1010	_
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		1		8.4	
HCM LOS					Α	
NAIssaul and INA in the		UDL 4	ГОТ	EDD	MDI	MOT
Minor Lane/Major Mvmt	ſ	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		1054	-		1592	-
HCM Lane V/C Ratio		0.003	-	-	0.001	-
HCM Control Delay (s/ve	h)	8.4	-	-	7.3	0
HCM Lane LOS		Α	-	-	Α	Α
HCM 95th %tile Q (veh)		0	-	-	0	-

Intersection						
Int Delay, s/veh	0.9					
		EDT	WDT	WDD	CDI	CDD
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	^	4	\$	4	Y	^
Traffic Vol, veh/h	0	24	10	1	4	0
Future Vol, veh/h	0	24	10	1	4	0
Conflicting Peds, #/hr	_ 0	_ 0	_ 0	_ 0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	26	11	1	4	0
Major/Minor N	Major1	N	Major2		Minor2	
Conflicting Flow All	12	0	viaj012 -	0	38	12
		U			12	
Stage 1	-	-	-	-	26	-
Stage 2	4 40	-	-	-		-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-		3.318
Pot Cap-1 Maneuver	1607	-	-	-	974	1069
Stage 1	-	-	-	-	1011	-
Stage 2	-	-	-	-	997	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1607	-	-	-	974	1069
Mov Cap-2 Maneuver	-	-	-	-	974	-
Stage 1	-	-	-	-	1011	-
Stage 2	-	-	-	-	997	-
Approach	EB		WB		SB	
HCM Control Delay, s/\			0		8.7	
HCM LOS	v 0		U		Α	
I ICIVI LOS					Α	
Minor Lane/Major Mvm	t	EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1607	-	-	-	974
		-	-	-	-	0.004
HCM Lane V/C Ratio						0.7
	veh)	0	-	-	-	8.7
HCM Lane V/C Ratio	veh)	0 A	-	-	-	8.7 A
HCM Lane V/C Ratio HCM Control Delay (s/v	veh)					

Intersection								
Int Delay, s/veh	17.9							
Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	W	LDIN	NDL	4	<u>351</u>	ODIN		
Traffic Vol, veh/h	141	14	10	877	369	49		
future Vol, veh/h	141	14	10	877	369	49		
<u> </u>	141	14	10	0//	0	49		
Conflicting Peds, #/hr	•			Free	Free	Free		
Sign Control RT Channelized	Stop -	Stop None	Free	None				
Storage Length	0	None -			-	None		
reh in Median Storage		-	-	0	0			
	e, # 0 0			0	0			
Grade, % Peak Hour Factor	85	85	94	94	91	91		
	0	85 0	94	94		91		
leavy Vehicles, %					2 40E			
1vmt Flow	166	16	11	933	405	54		
lajor/Minor	Minor2	N	//ajor1		//ajor2			
onflicting Flow All	1389	434	460	0	//ajuiz -	0		
Stage 1	433	434	400	-	_	-		
Stage 2	956	<u>-</u>	_		_	_		
ritical Hdwy	6.4	6.2	4.1		_			
itical Hdwy Stg 1	5.4	- 0.2	7.1	_	_	_		
ritical Hdwy Stg 2	5.4	_						
ollow-up Hdwy	3.5	3.3	2.2	_	_	_		
ot Cap-1 Maneuver	~ 159	626	1112	_	-			
Stage 1	658	-	1112	_	_	_		
Stage 2	376		-	_	<u>-</u>	-		
latoon blocked, %	310	_			_	_		
lov Cap-1 Maneuver	~ 155	625	1111	-	_			
lov Cap-1 Maneuver		020	1111	-	-	-		
Stage 1	644	-	-	-	-	-		
Stage 2	376	-	_	_	_	-		
Staye 2	310	-	-	-	-	-		
pproach	EB		NB		SB			
HCM Control Delay, s/			0.1		0			
ICM LOS	F							
linor Lane/Major Mvn	nt	NBL	NBT I	EBLn1	SBT	SBR		
apacity (veh/h)		1111	-	166	-	-		
CM Lane V/C Ratio		0.01	-	1.099	-	-		
CM Control Delay (sa	/veh)	8.3	0	154.7	-	-		
CM Lane LOS		Α	Α	F	-	-		
CM 95th %tile Q (vel	h)	0	-	9.4	-	-		
lotes								
Volume exceeds ca	nacity	\$· De	lav evo	eeds 30)0s	+: Com	outation Not Defined	*: All major volume in platoor
. Volumo oxoceus da	puolty	ψ. υσ	ay UNL	Jours J	,00	50111	Jakation Not Delined	. Tili major volume in piatoor

Intersection						
Int Delay, s/veh	0.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
		EDK	INDL			SDR
Lane Configurations	M	15	22	4	♣	2
Traffic Vol, veh/h	2	15	33	897	403	2
Future Vol, veh/h	2	15	33	897	403	2
Conflicting Peds, #/hr		3	_ 3	_ 0	_ 0	_ 3
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-		-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storag		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	65	65	90	90	90	90
Heavy Vehicles, %	0	0	0	1	2	0
Mvmt Flow	3	23	37	997	448	2
Major/Minor	Minor2		/lajor1		/lajor2	
Conflicting Flow All	1526	455	453	0	-	0
Stage 1	452	-	-	-	-	-
Stage 2	1074	_	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	_	_	-
Pot Cap-1 Maneuver	131	609	1118	_	_	_
Stage 1	645	-	-	_	_	_
Stage 2	331	_	_	_	_	_
Platoon blocked, %	331	_	_	_	_	_
	. 101	606	1115	-		
Mov Cap-1 Maneuver		606	1115	-	-	-
Mov Cap-2 Maneuver		-	-	-	-	-
Stage 1	596	-	-	-	-	-
Stage 2	330	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s			0.3		0	
HCM LOS	B		0.5		U	
HOW LOS	D					
Minor Lane/Major Mvi	mt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1115	-	412	_	_
HCM Lane V/C Ratio		0.033		0.063	_	_
HCM Control Delay (s		8.3	0	14.3	_	_
HCM Lane LOS	o, vorij	Α	A	В	-	_
	h)	0.1	-	0.2		_
HCM 95th %tile Q (ve	#11)	U. I	-	0.2	-	-

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	14	6	14	21	3	5	132	18	1	58	4
Future Vol, veh/h	1	14	6	14	21	3	5	132	18	1	58	4
Peak Hour Factor	0.45	0.45	0.45	0.43	0.43	0.43	0.71	0.71	0.71	0.84	0.84	0.84
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	2	31	13	33	49	7	7	186	25	1	69	5
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay, s/veh	7.8			8.3			8.7			7.9		
HCM LOS	Α			Α			Α			Α		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	3%	5%	37%	2%	
Vol Thru, %	85%	67%	55%	92%	
Vol Right, %	12%	29%	8%	6%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	155	21	38	63	
LT Vol	5	1	14	1	
Through Vol	132	14	21	58	
RT Vol	18	6	3	4	
Lane Flow Rate	218	47	88	75	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.257	0.058	0.114	0.092	
Departure Headway (Hd)	4.239	4.496	4.632	4.414	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	849	798	775	814	
Service Time	2.253	2.519	2.653	2.432	
HCM Lane V/C Ratio	0.257	0.059	0.114	0.092	
HCM Control Delay, s/veh	8.7	7.8	8.3	7.9	
HCM Lane LOS	Α	Α	Α	Α	
HCM 95th-tile Q	1	0.2	0.4	0.3	

Synchro 11 Report HCM 6th AWSC Page 3 DCL Vanasse & Associates

Intersection						
Int Delay, s/veh	1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
	EBL	EDK	INDL			אמט
Lane Configurations Traffic Vol, veh/h	'Y' 11		10	4	♣	13
•		5	12	155	63	
Future Vol, veh/h	11	5	12	155	63	13
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	75	75	96	96	86	86
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	15	7	13	161	73	15
Major/Minor N	/linor2	N	/lajor1	N	Major2	
						^
Conflicting Flow All	268	81	88	0	-	0
Stage 1	81	-	-	-	-	-
Stage 2	187	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	726	985	1520	-	-	-
Stage 1	947	-	-	-	-	-
Stage 2	850	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	719	985	1520	-	-	-
Mov Cap-2 Maneuver	719	-	-	_	_	_
Stage 1	938	_	_	_	_	_
Stage 2	850	_	_		_	_
Jiaye Z	000	<u>-</u>	_	_	-	-
Approach	EB		NB		SB	
HCM Control Delay, s/v	9.7		0.5		0	
HCM LOS	A					
Minor Lane/Major Mvmt		NBL	NBT I	EBLn1	SBT	SBR
Capacity (veh/h)		1520	-	785	-	-
HCM Lane V/C Ratio		0.008	-	0.027	-	-
HCM Control Delay (s/v	eh)	7.4	0	9.7	-	-
HCM Lane LOS		Α	Α	Α	-	-
HCM 95th %tile Q (veh))	0	-	0.1	-	-
222 /200						

HCM 6th TWSC
DCL Vanasse & Associates

Intersection						
Int Delay, s/veh	5.7					
	EBT	EBR	WBL	WBT	NBL	NBR
		EDR	VVDL		NDL W	INDIX
Lane Configurations	þ	٥	10	<u>4</u>		E
Traffic Vol, veh/h	3	0	12	1	1	5
Future Vol, veh/h	3	0	12	1	1	5
Conflicting Peds, #/hr	0	_ 0	0	_ 0	0	0
5	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	38	38	60	60	75	75
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	8	0	20	2	1	7
Major/Minor Ma	-:1		/_:_\0		Aire and	
	ajor1		Major2		Minor1	
Conflicting Flow All	0	0	8	0	50	8
Stage 1	-	-	-	-	8	-
Stage 2	-	-	-	-	42	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	1625	-	964	1080
Stage 1	-	-	-	-	1020	-
Stage 2	_	-	-	-	986	-
Platoon blocked, %	_	_		_		
Mov Cap-1 Maneuver	_	_	1625	_	952	1080
Mov Cap-1 Maneuver	_	<u>-</u>	1020	<u>-</u>	952	-
		_	_		1020	-
Stage 1	-	-	-	-		
Stage 2	-	-	-	-	974	-
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		6.7		8.4	
HCM LOS	U		0.1		A	
TIOM EGO					,,	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		1056	-	-	1625	-
HCM Lane V/C Ratio		0.008	-	-	0.012	-
HCM Control Delay (s/ve	h)	8.4	-	-	7.2	0
HCM Lane LOS	•	Α	-	-	Α	Α
HCM 95th %tile Q (veh)		0	-	-	0	-

HCM 6th TWSC DCL Vanasse & Associates

Intersection						
Int Delay, s/veh	0.7					
		EDD	WDL	WDT	NDI	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	- ↑			र्	¥	
Traffic Vol, veh/h	19	0	3	27	0	2
Future Vol, veh/h	19	0	3	27	0	2
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	21	0	3	29	0	2
	ajor1		Major2		Minor1	
Conflicting Flow All	0	0	21	0	56	21
Stage 1	-	-	-	-	21	-
Stage 2	-	-	-	-	35	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1595	_	952	1056
Stage 1	_	_	-	_	1002	-
Stage 2	-	-	-	_	987	_
Platoon blocked, %	_	_		_	301	
Mov Cap-1 Maneuver			1595	_	950	1056
Mov Cap-1 Maneuver	_		1090	_	950	1030
Stage 1	_	-	-	-	1002	-
•	-	-	-	-		
Stage 2	-	-	-	-	985	-
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		0.7		8.4	
HCM LOS	•		V .,		A	
					, \	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		1056	-	-	1595	-
HCM Lane V/C Ratio		0.002	-	-	0.002	-
HCM Control Delay (s/ve	h)	8.4	-	-	7.3	0
HCM Lane LOS	7	Α	-	-	Α	A
HCM 95th %tile Q (veh)		0	-	-	0	_
, out a (1011)		•			,	

Intersection						
Int Delay, s/veh	0.6					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	LDL	4	₩ <u>₩</u>	WDIX	N/	אומט
Traffic Vol, veh/h	0	13	20	5	3	0
Future Vol, veh/h	0	13	20	5	3	0
<u> </u>	0	0	0	0	0	0
Conflicting Peds, #/hr						
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	14	22	5	3	0
Majar/Minar	Maia = 1		10:00		Min a rO	
	Major1		/lajor2		Minor2	
Conflicting Flow All	27	0	-	0	39	25
Stage 1	-	-	-	-	25	-
Stage 2	-	-	-	-	14	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1587	-	-	-	973	1051
Stage 1	-	_	_	_	998	_
Stage 2	_	-	-	_	1009	_
Platoon blocked, %		_	_	_		
Mov Cap-1 Maneuver	1587	_	_	_	973	1051
Mov Cap-2 Maneuver	-	<u>_</u>	_	_	973	-
Stage 1	_		_	_	998	_
	-	-	-	-	1009	_
Stage 2	-	-	-	_	1009	_
Approach	EB		WB		SB	
HCM Control Delay, s/	v 0		0		8.7	
HCM LOS					A	
				14/5-	14/5-	201 (
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR :	
Capacity (veh/h)		1587	-	-	-	973
HCM Lane V/C Ratio		-	-	-	-	0.003
HCM Control Delay (s/	veh)	0	-	-	-	8.7
HCM Lane LOS		Α	-	-	-	Α
HCM 95th %tile Q (veh	1)	0	-	-	-	0
,						